

**ADDENDUM NO.3**

**LOWER POPLAR WATER RECLAMATION FACILITY  
INFLUENT PUMP STATION IMPROVEMENTS**

**MACON WATER AUTHORITY  
MACON, GEORGIA**

**DRAWINGS**

Delete 09-S001 and replace with Drawing 09-S001 attached.

Delete Drawing 09-S102 and replace with Drawing 09-S102 attached

Delete Drawing 09-S103 and replace with Drawing 09-S103 attached

Delete Drawing 09-SD101 and replace with Drawing 09-SD101 attached.

Delete Drawing 09-SD102 and replace with Drawing 09-SD102 attached.

Delete Drawing 09-SD103 and replace with Drawing 09-SD103 attached.

Delete Drawing 09-S301 and replace with Drawing 09-S301 attached.

Delete Drawing 09-S302 and replace with Drawing 09-S302 attached.

Delete Drawing 09-S501 and replace with Drawing 09-S501 attached.

*Bidder Must Acknowledge Receipt of this Addendum on Bid Form*

August 14, 2024  
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**CODES AND STANDARDS**

THE FOLLOWING CODES AND STANDARDS HAVE BEEN USED AS THE BASIS FOR DESIGN AND/OR SHALL BE UTILIZED BY THE CONTRACTOR TO ESTABLISH MINIMUM LEVELS OF QUALITY AND CONSTRUCTION TECHNIQUES.

- 1. GENERAL
A. INTERNATIONAL BUILDING CODE (IBC 2018) WITH GEORGIA STATE AMENDMENTS.
B. AMERICAN SOCIETY OF CIVIL ENGINEERS, "MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES," (ASCE 7-16).
2. CONCRETE
A. AMERICAN CONCRETE INSTITUTE, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318-14)
B. AMERICAN CONCRETE INSTITUTE, "SPECIFICATIONS FOR STRUCTURAL CONCRETE," (ACI 301-16)
C. AMERICAN CONCRETE INSTITUTE, "GUIDE TO CONCRETE FLOOR AND SLAB CONSTRUCTION" (ACI 302.1R-15)
D. AMERICAN CONCRETE INSTITUTE, "CODE REQUIREMENTS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES" (ACI 350-06)
3. STRUCTURAL STEEL
A. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "STEEL CONSTRUCTION MANUAL," FIFTEENTH EDITION
B. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," (ANSI/AISC 360-16).
C. AMERICAN WELDING SOCIETY, "STRUCTURAL WELDING CODE-STEEL" (AWS D1.1-2018).

**DESIGN CRITERIA**

THE STRUCTURE HAS BEEN DESIGNED FOR THE FOLLOWING LOADS.

- 1. DEAD LOADS: ACTUAL WEIGHTS OF BUILDING MATERIALS, STRUCTURAL COMPONENTS, AND EQUIPMENT.
A. ROOF DEAD LOADS (PEMB CANOPY)
1. PEMB SUPERSTRUCTURE ACTUAL 8 PSF
2. MPE UTILITIES / COLLATERAL
B. PUMP STATION SLAB LOAD
1. EXISTING CONCRETE SELF-WEIGHT 32 PSF
2. NEW 4" LW TOPPING
2. LIVE LOADS
A. ROOF LIVE LOADS (PEMB CANOPY) 20 PSF (REDUCIBLE)
B. FLOOR LIVE LOADS
1. PUMP STATION SLAB LOAD (ALL LEVELS) 60 PSF
C. MISCELLANEOUS LIVE LOADS
1. GUARDRAILS/HANDRAILS
a. 50 PLF FOR AREAS W/OCCUPANT LOAD GREATER THAN OR EQUAL TO 50.
b. OR 20 PLF FOR AREAS W/OCCUPANT LOAD LESS THAN 50.
c. OR 200 LB CONCENTRATED LOAD APPLIED IN ANY DIRECTION AT ANY POINT.
2. LADDERS (FIXED): 300 LB CONCENTRATED LOAD FOR EVERY 10 FT OF HEIGHT.
3. SNOW LOADS
A. GROUND SNOW LOAD (P\_g) 10 PSF
B. THERMAL FACTOR (C\_t) 1.2
C. EXPOSURE FACTOR (C\_e) 0.9
D. IMPORTANCE FACTOR (I\_s) 1.1
E. SLOPE FACTOR (C\_s) 1.0
F. BALANCED SNOW LOAD 8.3 PSF
G. RAIN-ON-SNOW SURCHARGE 0.0 PSF
H. DESIGN UNIFORM SNOW LOAD (P\_d) 11.0 PSF
4. WIND LOADS
A. BUILDING
1. ULTIMATE DESIGN WIND SPEED (V\_u) 120 MPH
2. ALLOWABLE STRESS DESIGN WIND SPEED (V ASD) 93 MPH
3. RISK CATEGORY III
4. EXPOSURE CATEGORY C
5. INTERNAL PRESSURE COEFF. (GC\_p) +/- 0.18
5. SEISMIC LOADS
A. BUILDING
1. RISK CATEGORY III
2. SEISMIC IMPORTANCE FACTOR (I\_a) 1.25
3. 0.2 SEC MAPPED SPECTRAL ACCELERATION (S\_s) 0.185
4. 1.0 SEC MAPPED SPECTRAL ACCELERATION (S\_1) 0.077
5. SITE CLASS E
6. 0.2 SEC DESIGN SPECTRAL ACCELERATION (S\_DS) 0.297
7. 1.0 SEC DESIGN SPECTRAL ACCELERATION (S\_D1) 0.216
8. SEISMIC DESIGN CATEGORY D
9. BASIC SEISMIC FORCE RESISTING SYSTEM INTERMEDIATE STEEL MOMENT FRAMES (CANOPY)
10. DESIGN BASE SHEAR 0.01xW
11. SEISMIC RESPONSE COEFFICIENT (C\_d) 0.01
12. RESPONSE MODIFICATION COEFFICIENT (R) 4.5
13. ANALYSIS PROCEDURE USED EQUIVALENT LATERAL FORCE PROCEDURE
6. RAIN LOADS
A. RAINFALL INTENSITY RATE (100-YEAR) 3.9 IN/HR

**FOUNDATIONS**

- 1. DEEP FOUNDATION AND SPECIALTY FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS REPORTED IN THE SITE SPECIFIC GEOTECHNICAL EXPLORATION REPORT PREPARED BY TERRACON, DATED OCTOBER 4, 2023. THE CONTRACTOR SHALL OBTAIN A COPY OF THE REPORT FOR REVIEW AND REFERENCE.
2. PER THE GEOTECHNICAL REPORT, ESTIMATED HELICAL ANCHOR DEPTH ASSUMED TO BE 25 FEET BELOW GRADE, EXCEPT FOR THE PIPE VALVE SUPPORT FOUNDATIONS DIRECTLY ADJACENT TO THE EXISTING PUMP STATION (15 FEET MAX). SEE DETAIL S109-S501. FINAL ANCHOR LENGTHS TO BE DETERMINED BY THE HELICAL ANCHOR INSTALLER'S ENGINEER OF RECORD.
3. A MINIMUM OF SIX (6) COMPRESSION LOAD TESTS SHALL BE CONDUCTED PER ASTM D1143 SPECIFICATIONS. CONTRACTOR TO PROVIDE A LOAD TEST PLAN PRIOR TO CONSTRUCTION FOR REVIEW AND APPROVAL BY THE ENGINEER OF RECORD. TEST RESULTS SHALL BE PROVIDED TO THE ENGINEER OF RECORD.
4. CONTRACTOR SHALL KEEP ALL FREE STANDING WATER OUT OF EXCAVATION. CONTRACTOR SHALL PROVIDE DEWATERING MEASURES AS NECESSARY PRIOR TO PLACING CONCRETE.
5. EXISTING SOIL WHICH IS DEEMED NON-USABLE BY THE GEOTECHNICAL ENGINEER DUE TO FAILURE OF THE CONTRACTOR TO PROMPTLY DE-WATER THE SITE SHALL BE REMOVED AND REPLACED WITH SUITABLE FILL AT THE CONTRACTOR'S EXPENSE. DESIGN OF TEMPORARY AND PERMANENT SHORING FOR EXCAVATIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
7. FOR WALLS OR GRADE WALLS HAVING FILL ON EACH SIDE, PROCEED WITH BACKFILLING OPERATIONS SIMULTANEOUSLY IN UNIFORM LIFTS. DIFFERENTIAL ELEVATION OF TOP OF LIFTS BETWEEN EACH SIDE SHALL NOT EXCEED 18 INCHES.

**CONCRETE**

- 1. MINIMUM 28 DAY CONCRETE COMPRESSIVE STRENGTH SHALL BE AS FOLLOWS:
A. MAT FOUNDATIONS 4,800 PSI
B. PIPE SUPPORTS 4,500 PSI
C. ELECTRICAL ROOM FLOOR SLABS 4,500 PSI
D. PUMPHOUSE TOPPING SLAB & WALLS 4,500 PSI
2. CONCRETE SHALL BE PROPORTIONED, BATCHED, MIXED, PLACED, CONSOLIDATED, AND CURED IN ACCORDANCE WITH ACI 301, 304, 308, 309 AND 318.
3. ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED.
4. PUMPHOUSE TOPPING SLAB SHALL MEET THE FOLLOWING SPECIFICATIONS:
A. TYPE K CEMENT
B. FORTA MACRO SYNTHETIC FIBERS AT A DOSAGE RATE OF 7.5 POUNDS PER CUBIC YARD. COORDINATE WITH FIBER MANUFACTURER PRIOR TO PLACEMENT OF CONCRETE
5. WHERE STRIP/GRADE FOOTINGS OR WALLS INTERSECT COLUMN FOUNDATIONS, LONGITUDINAL REINFORCEMENT SHALL BE CONTINUOUS THROUGH THE COLUMN FOUNDATION.
6. UNLESS OTHERWISE SHOWN, THE CONCRETE CLEAR COVER AT ALL REINFORCING STEEL SHALL BE:
A. CONCRETE CAST AGAINST EARTH 3"
B. CONCRETE EXPOSED TO EARTH OR WEATHER 2"
C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER 2"
7. ALL CONCRETE SHALL BE MECHANICALLY VIBRATED IN ACCORDANCE WITH ACI 304 AND ACI 309.
8. PROVIDE A 3/4"x45 DEGREE CHAMFERED CORNERS AT ALL EXPOSED CONCRETE CORNERS UNO.

**SLAB ON GRADE**

- 1. THE GEOTECHNICAL ENGINEER SHALL REVIEW THE AGGREGATE BASE AND VERIFY A MINIMUM MODULUS OF SUBGRADE REACTION OF 100 PCI HAS BEEN ACHIEVED.
2. FLOOR SLABS TO BE SUPPORTED BY A MINIMUM OF 12 INCHES OF APPROVED ON-SITE OR IMPORTED SOILS PLACED AND COMPACTED AS SPECIFIED IN THE GEOTECHNICAL EXPLORATION REPORT.
3. PROVIDE A 6" COMPACTED GRANULAR SUB-BASE ON TOP OF COMPACTED FILL.
4. EXCAVATED / STRIPPED AREAS SHALL BE PROOF-ROLLED WITH APPROPRIATE EQUIPMENT AS APPROVED BY THE GEOTECHNICAL ENGINEER. SOFT AREAS SHALL BE REMOVED AND REPLACED WITH APPROVED BACKFILL AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
5. SAWED CONTROL JOINTS SHALL BE CUT AS SOON AS SLAB CAN BE WALKED ON, BUT STARTED NO LATER THAN 8 HOURS AFTER POURING. CONTROL JOINTS SHALL BE COMPLETED NO LATER THAN 16 HOURS AFTER POURING. THESE TIME LIMITS SHALL APPLY REGARDLESS OF THE TIME OF DAY. AN EARLY ENTRY DRY CUT SAW SUCH AS THE SOFF-CUT SYSTEM SHALL BE USED.
6. PROVIDE ADDITIONAL REINFORCING IN TOP FACE OF SLAB AT ALL RE-ENRANT CORNERS AND DOOR OPENINGS
7. ADEQUATE MEASURE TO PREVENT PLASTIC SHRINKAGE OF SLAB SHALL BE TAKEN BY THE CONTRACTOR AS OUTLINED IN ACI 302.1R.

**CONCRETE/CMU ANCHORS**

- 1. REFER TO SPEC SECTION 05 50 00 METAL FABICATIONS FOR ADDITIONAL REQUIREMENTS AND MATERIAL TYPE.
2. SUBSTITUTION OF EXPANSION OR DRILLED AND GROUTED-IN ANCHORS FOR EMBEDDED ANCHORS SHOWN ON THE DRAWINGS WILL NOT BE PERMITTED UNLESS APPROVED BY THE ENGINEER.
3. CARE SHALL BE TAKEN WHEN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH REINFORCING WHERE POSSIBLE. HOLES SHALL BE DRY, HAMMER DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ALTERNATIVE DRILLING METHODS AND INSTALLATION CONDITIONS MAY BE ACCEPTABLE PROVIDED INSTALLER HAS OBTAINED WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER OR RECORD.
4. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE.
5. PRIOR TO COMMENCEMENT OF WORK, THE CONTRACTOR SHALL ARRANGE FOR A MANUFACTURER'S FIELD REPRESENTATIVE TO PROVIDE INSTALLATION TRAINING FOR ALL PRODUCTS TO BE USED. ONLY TRAINED INSTALLERS SHALL PERFORM POST-INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND BE MADE AVAILABLE TO THE EOR AS REQUESTED.
6. EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES AS PROVIDED BY HILTI, INC.
A. ANCHORAGE TO CONCRETE
a. ADHESIVE (EPOXY) ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:
1. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HIT-Z-R 316 SS ROD.
2. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM WITH HAS-316 SS THREADED ROD PER ICC ESR-4868.
b. MECHANICAL (EXPANSION) ANCHORS FOR CRACKED AND UNCRACKED CONCRETE
1. HILTI KWIK BOLT-T22 SS 316 EXPANSION ANCHORS PER ICC ESR-4266
2. HILTI KWIK HUS-EZ SS 316 SCREW ANCHORS PER ICC ESR-3027
B. REBAR ANCHORAGE TO CONCRETE
a. ADHESIVE FOR CRACKED AND UNCRACKED CONCRETE USE:
1. HILTI HIT-HY 500 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM.
C. ANCHORAGE TO SOLID GROUTED MASONRY
a. ADHESIVE (EPOXY) ANCHORS USE:
1. HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM WITH HAS 316 SS THREADED ROD.
2. MECHANICAL (EXPANSION) ANCHORS USE:
3. HILTI KWIK BOLT-T22 SS 316 EXPANSION ANCHORS PER ICC ESR-4561
4. HILTI KWIK HUS-EZ SS 316 SCREW ANCHORS PER ICC ESR-3056
D. ANCHORAGE TO HOLLOW / MULTI-WYTHE MASONRY
a. ADHESIVE ANCHORS USE:
1. HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM WITH HAS 316 SS THREADED ROD.
2. HAS APPROVED TIE SIZE SCREEN TUBE SHALL BE USED PER ADHESIVE MANUFACTURER'S RECOMMENDATION.
7. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI OR OTHER SUCH METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OR RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS THAT HAVE BEEN SEALED BY ANOTHER LICENSED ENGINEER DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE TO MEETING THE PERFORMANCE OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, INSTALLATION TEMPERATURE, MOISTURE CONDITION OF CONCRETE, AND DRILLING METHODS.

**REINFORCING STEEL FOR CONCRETE**

- 1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 (DEFORMED).
2. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064 AND SHALL BE PROVIDED IN FLAT SHEETS ONLY. FABRIC SHALL LAP TWO FULL MESHES AND BE SECURELY FASTENED AT EACH SIDE AND EACH END.
3. DETAILING, FABRICATION, AND ERECTION OF REINFORCING STEEL, UNLESS OTHERWISE NOTED, SHALL CONFORM TO ACI MNL-88, THE CRSI, "MANUAL OF STANDARD PRACTICE," AND ACI 318.
4. REINFORCING STEEL SHALL BE CONTINUOUS ACROSS ALL CONSTRUCTION JOINTS UNO.
5. REINFORCING STEEL SHALL NOT BE HEATED OR WELDED AND MUST BE DRY AND FREE OF CONTAMINANTS SUCH AS RUST, DIRT, GREASE, AND PROTECTIVE COATINGS.
6. ALL BAR SPLICES SHALL BE CLASS B TENSION SPLICES IN ACCORDANCE WITH ACI 318.

**MISCELLANEOUS**

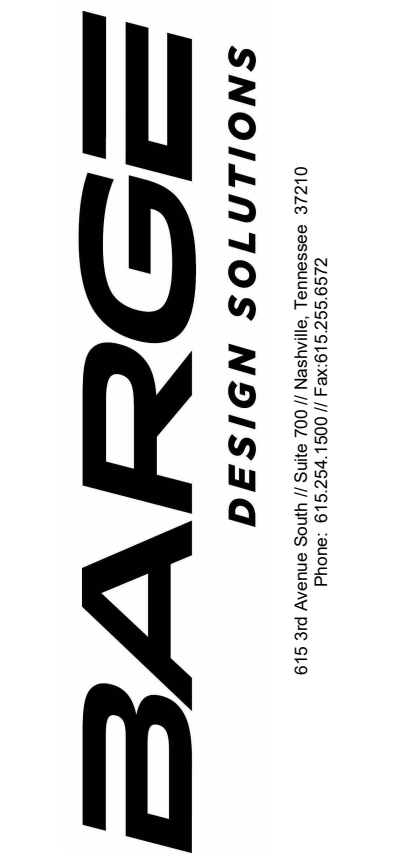
- 1. GENERAL NOTES AND TYPICAL DETAILS DESCRIBE GENERAL CRITERIA APPLICABLE TO ALL SIMILAR CONDITIONS THROUGHOUT THE PROJECT REGARDLESS OF WHETHER OR NOT THEY ARE SPECIFICALLY REFERENCED IN THE PLANS OR DETAILS.
2. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION, THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING CLARIFICATION FROM THE STRUCTURAL ENGINEER BEFORE CONTINUING WITH CONSTRUCTION.
3. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY.
4. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS, FOR DIMENSIONS TO BE CONFIRMED AT THE JOBSITE, FOR FABRICATION PROCESSES, AND FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION.
5. NO SUBSTITUTIONS OF MATERIAL WILL BE ALLOWED WITHOUT WRITTEN PERMISSION FROM THE ENGINEER.
6. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, MILL CERTIFICATES, AND PRODUCT DATA FOR ALL MATERIALS AND PRODUCTS SHOWN IN THE CONSTRUCTION DOCUMENTS, INCLUDING BUT NOT LIMITED TO, CONCRETE MIX DESIGNS, STEEL REINFORCEMENT, AND CAST-IN-PLACE AND POST-INSTALLED ANCHORS. THE SHOP DRAWINGS SHALL INCLUDE DETAIL FABRICATION AND ERECTION DRAWINGS AND SHALL CONTAIN PLANS, ELEVATIONS, AND DETAILS. REPRODUCTION OF THE CONSTRUCTION DRAWINGS IS NOT AN ACCEPTABLE SHOP DRAWING SUBMITTAL. SHOP DRAWINGS SHALL NOT BE REVIEWED FOR APPROVAL UNLESS CHECKED BY THE FABRICATOR AND APPROVED BY THE CONTRACTOR. REVIEW OF THE SHOP DRAWINGS BY THE ENGINEER DOES NOT ELIMINATE THE CONTRACTOR'S RESPONSIBILITY TO COMPLY WITH ALL REQUIREMENTS SET FORTH IN THE CONSTRUCTION DOCUMENTS.
8. CONTRACTOR SHALL COMPLY WITH LOCAL, STATE, FEDERAL AND OWNER'S SAFETY REGULATIONS WHILE WORKING. STRUCTURAL ENGINEER DOES NOT ASSUME ANY RESPONSIBILITY FOR CONSTRUCTION SITE SAFETY.
9. CONTRACTOR SHALL REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS
10. VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, AND ELEVATIONS BEFORE STARTING WORK. NOTIFY STRUCTURAL ENGINEER OF ANY DISCREPANCY. NOTIFY ARCHITECTURAL ENGINEER IN WRITING OF ANY DISCREPANCY IN THE FIELD CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS.

**PRE-ENGINEERED METAL BUILDING**

- 1. DESIGN OF STRUCTURE SHALL BE IN ACCORDANCE WITH THE "CODES AND STANDARDS" AND "DESIGN CRITERIA" AS LISTED ON THIS DRAWING.
2. THE METAL BUILDING MANUFACTURER SHALL BE SOLELY RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THE SUPERSTRUCTURE INCLUDING PURLINS, RIGID FRAMES, COLUMNS, GIRTS, BASEPLATES, X-BRACES, AND ANCHOR BOLTS (EXCLUDING EMBEDMENT). A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF GEORGIA SHALL DESIGN THE MEMBERS OR DIRECTLY SUPERVISE THE DESIGN AND AFFIX HIS SEAL TO ALL DRAWINGS AND DESIGN CALCULATIONS.
3. THE METAL BUILDING MANUFACTURER SHALL BE RESPONSIBLE FOR THE ANCHOR BOLT DESIGN, INCLUDING QUANTITY, DIAMETER, AND MATERIAL TYPE TO ADEQUATELY TRANSFER BUILDING COLUMN REACTIONS TO THE FOUNDATION. MINIMUM EMBEDMENT LENGTHS SHALL BE AS SHOWN ON THE FOUNDATION DRAWINGS. THE GENERAL CONTRACTOR SHALL PROVIDE THE ANCHOR BOLTS SPECIFIED.
4. CONTRACTOR SHALL VERIFY QUANTITY AND PLACEMENT LOCATIONS OF ANCHOR BOLTS WITH METAL BUILDING MANUFACTURER. ANCHOR BOLTS MUST BE LOCATED BY MEANS OF A TEMPLATE. DO NOT HAND SET ANCHOR BOLTS. ANCHOR BOLT LAYOUT, DIAMETER, PROJECTION, AND MATERIAL SHALL BE AS SHOWN ON THE METAL BUILDING DRAWINGS.
5. ANCHOR BOLT EMBEDMENT SHALL BE AS INDICATED ON THE FOUNDATION DRAWINGS.
6. THE METAL BUILDING COLUMNS SHALL HAVE PINNED BASES AND SHALL TRANSFER NO MOMENTS TO THE FOUNDATIONS.
7. HORIZONTAL DEFLECTION OF THE RIGID FRAMES AND BRACED FRAMES SHALL NOT EXCEED H/120 UNDER ALL LOAD COMBINATIONS USING SERVICE LEVEL WIND LOADS. REFER TO MECHANICAL DRAWINGS, ELECTRICAL DRAWINGS, AND EQUIPMENT VENDOR DRAWINGS FOR EQUIPMENT TO BE SUPPORTED BY PRE-ENGINEERED COMPONENTS AND OPENINGS WHICH REQUIRE SPECIAL FRAMING. PROVIDE ANY ADDITIONAL PURLINS, GIRTS, ETC. AS REQUIRED FOR THESE ITEMS.
9. ALL BOLTED CONNECTIONS SHALL HAVE AT LEAST TWO BOLTS.
10. NO FIELD MODIFICATIONS SHALL BE MADE TO ANY PRIMARY OR SECONDARY STRUCTURAL MEMBER EXCEPT AS AUTHORIZED IN WRITING BY BUILDING MANUFACTURER DESIGN ENGINEER AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
11. THE FOUNDATIONS HAVE BEEN DESIGNED FOR ESTIMATED COLUMN AND FRAME REACTIONS. PRIOR TO FABRICATION AND PRIOR TO ANY FOUNDATION WORK, THE ACTUAL COLUMN AND FRAME REACTIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. IF, IN THE OPINION OF THE ENGINEER, THE ACTUAL REACTIONS DIFFER APPRECIABLY FROM THE ESTIMATED, THE ENGINEER SHALL REDESIGN THE FOUNDATION FOR THE ACTUAL REACTIONS.

**STRUCTURAL STEEL**

- 1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING UNLESS OTHERWISE SHOWN:
WIDE FLANGE AND WT SHAPES ASTM A992, UNO
S SHAPES, CHANNELS, ANGLES, & PLATES ASTM A36, UNO
SMOOTH ROD ASTM A36
THREADED ROD ASTM A36
HSS, RECTANGULAR OR SQUARE ASTM A500 GR. C, 50 KSI
STEEL PIPE ASTM A53, GR. B
ANCHOR RODS ASTM F1554, GR AS INDICATED
HIGH STRENGTH BOLTS ASTM A325 OR ASTM 490
TWIST OFF TENSION CONTROL BOLTS ASTM F1952 FOR A325 BOLTS AND F2280 FOR A490 BOLTS
HARDENED WASHERS ASTM F438
DIRECT TENSION INDICATOR WASHERS ASTM F959
HEAVY HEX NUTS ASTM A563
ROLLED STEEL FLOOR PLATE ASTM A786
STAINLESS STEEL SHAPES AND PLATE ASTM A276
STAINLESS STEEL BOLTS ASTM F959, TYPE 316
WELDING ELECTRODES AWS A5.1 OR A5.5 E-70XX
ELECTRODES WITH CHARPY V-NOTCH (CVN) TEST VALUES OF A MINIMUM 20 FOOT-POUNDS AT -20 DEGREES FAHRENHEIT ARE TO BE USED AT THE FOLLOWING LOCATIONS:
i. COMPLETE JOINT PENETRATION WELDS
ii. BEAM TO COLUMN MOMENT CONNECTIONS – INCLUDING FLANGE, WEB AND CONTINUITY PLATE FILLET AND PARTIAL JOINT PENETRATION WELDS
iii. BRACE CONNECTIONS – INCLUDING BRACE, GUSSET, BASE PLATES, BEAM STIFFENER PLATES, AND CONTINUITY PLATE FILLET AND PARTIAL JOINT PENETRATION WELDS
iv. WELD NOTED "CVN" ON THE DRAWINGS
2. STRUCTURAL STEEL DESIGN, DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO:
1. AISC, "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
2. AISC, "CODE OF STANDARD PRACTICE", INCLUDING COMMENTARY
3. AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 AND A490 BOLTS"
3. WELDING SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE" AND BE PERFORMED BY CERTIFIED WELDERS USING E70XX WELDING ELECTRODES.
4. REMOVE RUST, DIRT, PAINT AND GALVANIZING FROM STEEL PRIOR TO WELDING.
5. WELDS SHOWN ON STRUCTURAL DRAWINGS ARE MINIMUM DESIGN REQUIREMENTS. USE THE MINIMUM WELD SIZE PER AISC WHERE WELD SIZE IS NOT INDICATED. THE FABRICATOR'S SHOP DRAWINGS SHALL REFLECT WELDS IN ACCORDANCE WITH AWS / AISC REQUIREMENTS.
6. ALL GROOVE WELDS TO BE COMPLETE PENETRATION.
7. CONNECTIONS NOT SPECIFICALLY DETAILED IN THE CONTRACT DOCUMENTS SHALL BE DESIGNED, COORDINATED AND INSPECTED BY A PROFESSIONAL STRUCTURAL ENGINEER LICENSED IN THE PROJECT STATE. SUBMIT SIGNED AND SEALED CALCULATIONS AS A FORMAL SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD. PER THE AISC CODE OF STANDARD PRACTICE, PROVIDE CORRELATION BETWEEN CALCULATIONS AND CONNECTIONS SHOWN ON THE STRUCTURAL STEEL SHOP DRAWINGS.
8. BOLTED CONNECTIONS SHALL BE BEARING TYPE WITH THREADS INCLUDED IN THE SHEAR PLANE.
9. UNLESS NOTED OTHERWISE, MINIMUM BOLT SIZE IS 3/4" DIAMETER STAINLESS STEEL GRADE F593, TYPE 316. INSTALL HIGH STRENGTH BEARING BOLTS TO A SNUG FIT CONDITION AS DEFINED BY AISC. LOCK WASHERS AND LOCK NUTS ARE STRICTLY PROHIBITED.
10. SEQUENCE DRIVEN SHARED CONNECTIONS WILL NOT BE PERMITTED AND MAY BE UNSAFE DURING THE CONNECTION PROCESS UNDER CERTAIN CONDITIONS. PROVIDE STAGGERED CLIP ANGLES, ERECTION SEATS ON BOTH SIDES OF COLUMN WEBS, OR SHEAR TAB TYPE CONNECTIONS IN COMPLIANCE WITH OSHA 1926 SUBPART R TO ALLOW FOR MEMBERS TO BE INSTALLED FROM EITHER DIRECTION REGARDLESS OF SEQUENCE.
11. BEAM CONNECTIONS SHALL BE STANDARD, SIMPLE SHEAR CONNECTIONS WITH DOUBLE FRAMING ANGLES UNO. IN NO CASE SHALL THE LENGTH OF THE FRAMED CONNECTION BE LESS THAN ONE-HALF OF THE "T" DIMENSION OF THE BEAM WEB.
12. CONNECTION ANGLES SHALL BE 5/16" MINIMUM THICKNESS.
13. MINIMUM BOLTED CONNECTION SHALL BE AS FOLLOWS:
1. DEPTH: 6" - 10" USE 2 ROWS OF BOLTS
2. DEPTH: 12" - 18" USE 3 ROWS OF BOLTS
3. DEPTH: 21" - 24" USE 4 ROWS OF BOLTS
4. DEPTH: 27" - 30" USE 5 ROWS OF BOLTS
5. DEPTH: 33" - 40" USE 6 ROWS OF BOLTS
14. BEAM REACTIONS ARE SHOWN ON THE DRAWINGS IN LRFD FORMAT. IN CASES WHERE NO REACTIONS ARE PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR A MINIMUM FORCE OF 15 KIPS, UNO.
15. ALL BOLTED CONNECTION HOLES ARE TO BE STANDARD HOLES. SHORT SLOTTED HOLES ARE PERMITTED AS LONG AS THERE IS NO FORCE IN THE DIRECTION OF THE SLOT (I.E. HOLE DIA. = BOLT DIA. + 1/16").
16. BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED SO THAT ALL FORCE COMPONENTS ARE DELIVERED DIRECTLY TO THE INTERSECTION OF THE WORKLINES OF THE MEMBERS. WHERE THIS IS NOT POSSIBLE OR PRACTICAL, CONNECTIONS SHALL BE DESIGNED TO ACCOUNT FOR THE RESULTING ECCENTRICITIES.
17. SWAY FRAMES, X-BRACING, LACING AND SIMILAR TYPE MEMBERS SHALL EITHER DEVELOP THE AXIAL FORCE INDICATED ON THE DRAWINGS OR THE ALLOWABLE TENSION FORCE IN THE MEMBER WHERE NO FORCES ARE SHOWN. THERE SHALL BE A MINIMUM OF TWO BOLTS PER CONNECTION.
18. AXIAL FORCES IN MEMBERS ARE SHOWN AS FOLLOWS:
1. (+) INDICATES TENSION IN MEMBER.
2. (-) INDICATES COMPRESSION IN MEMBER.
19. BUILT UP MEMBERS SHALL HAVE STITCH PLATES COMPLYING WITH AISC REQUIREMENTS. TENSION MEMBERS SHALL HAVE AT LEAST ONE STITCH PLATE LOCATED AT MID-LENGTH AND BUILT UP COMPRESSION MEMBERS SHALL HAVE AT LEAST TWO STITCH PLATES LOCATED AT THIRD POINTS OR A MAXIMUM OF 5'-0" OC SPACING, WHICHEVER IS LESS. ASSUME BUILT UP MEMBERS ARE COMPRESSION MEMBERS UNLESS NOTED OTHERWISE ON DRAWINGS.
20. STEEL SURFACES THAT ARE TO RECEIVE SPRAYED ON FIREPROOFING, SCHEDULED TO RECEIVE SHEAR STUDS OR WILL BE WELDED/BOLTED SHALL NOT BE PAINTED.
21. NO OPENINGS SHALL BE CUT IN STRUCTURAL MEMBERS UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DRAWINGS.
22. THE STRUCTURE IS DESIGNED FOR A COMPLETED CONDITION ONLY AND THEREFORE MAY REQUIRE ADDITIONAL SUPPORT TO MAINTAIN STABILITY BEFORE COMPLETION.
23. ALL EXTERIOR STEEL FOR THE CANOPY STRUCTURE, INCLUDING BOLTS AND GUARDRAIL SHALL BE HOT-DIPPED GALVANIZED. ANCHOR BOLTS SHALL BE STAINLESS STEEL. REPAIR DAMAGED GALVANIZING AND FIELD WELDS WITH GALVANIZING REPAIR PAINT (ZRC GALVALIITE, OR EQUAL).
24. STAIRS SHOWN ON PLAN AND ARCH SHALL BE A DELEGATED DESIGN WITH EXCEPTION OF LATERAL FORCE RESISTING SYSTEM AND POSTS. CALCULATIONS SEALED BY AN ENGINEER REGISTERED IN THE PROJECT STATE SHALL BE SUBMITTED WITH THE SHOP DRAWINGS.

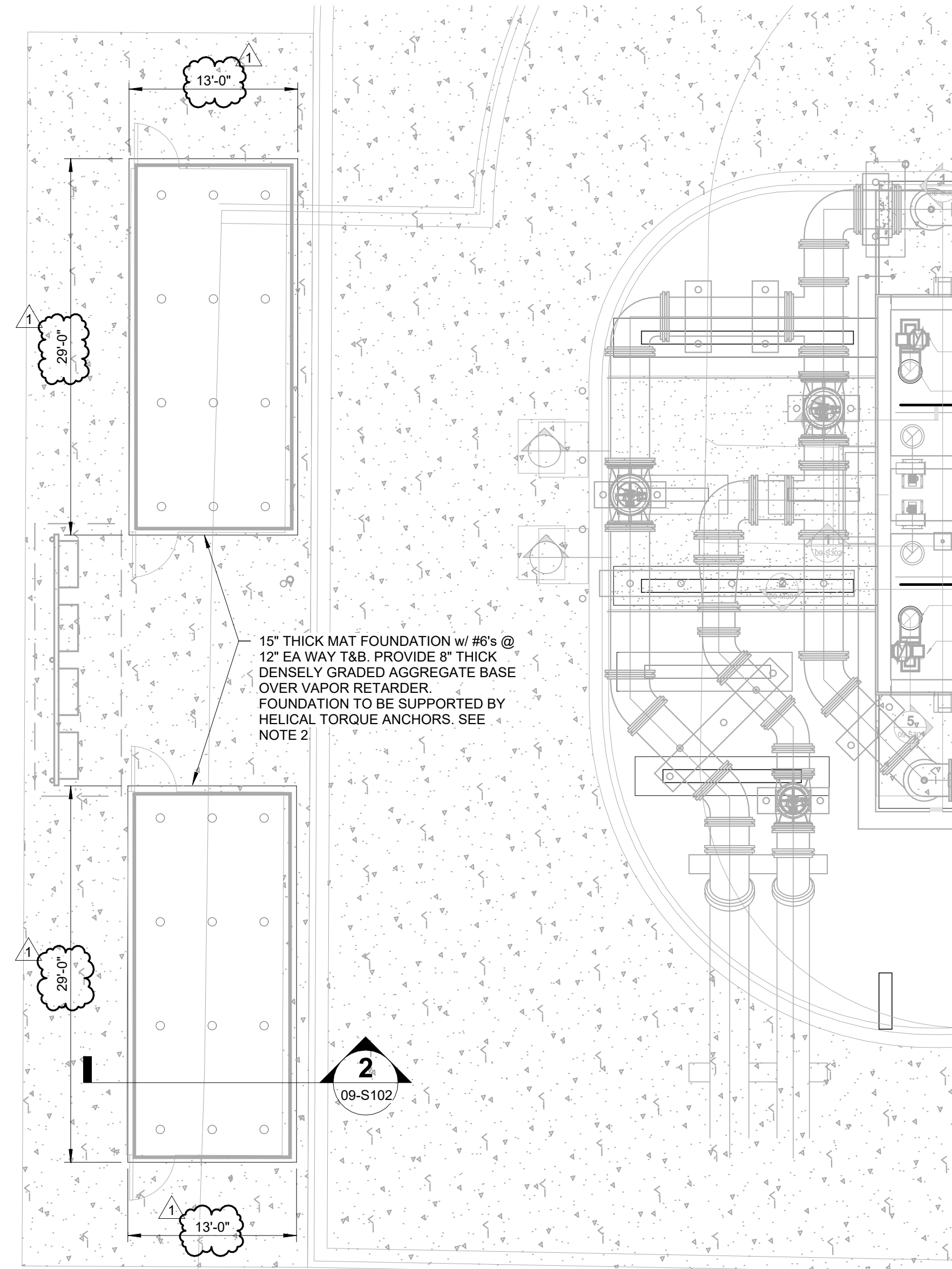


GENERAL NOTES
LOWER POPLAR WATER RECLAMATION FACILITY
INFLUENT PUMP STATION IMPROVEMENTS
MACON WATER AUTHORITY

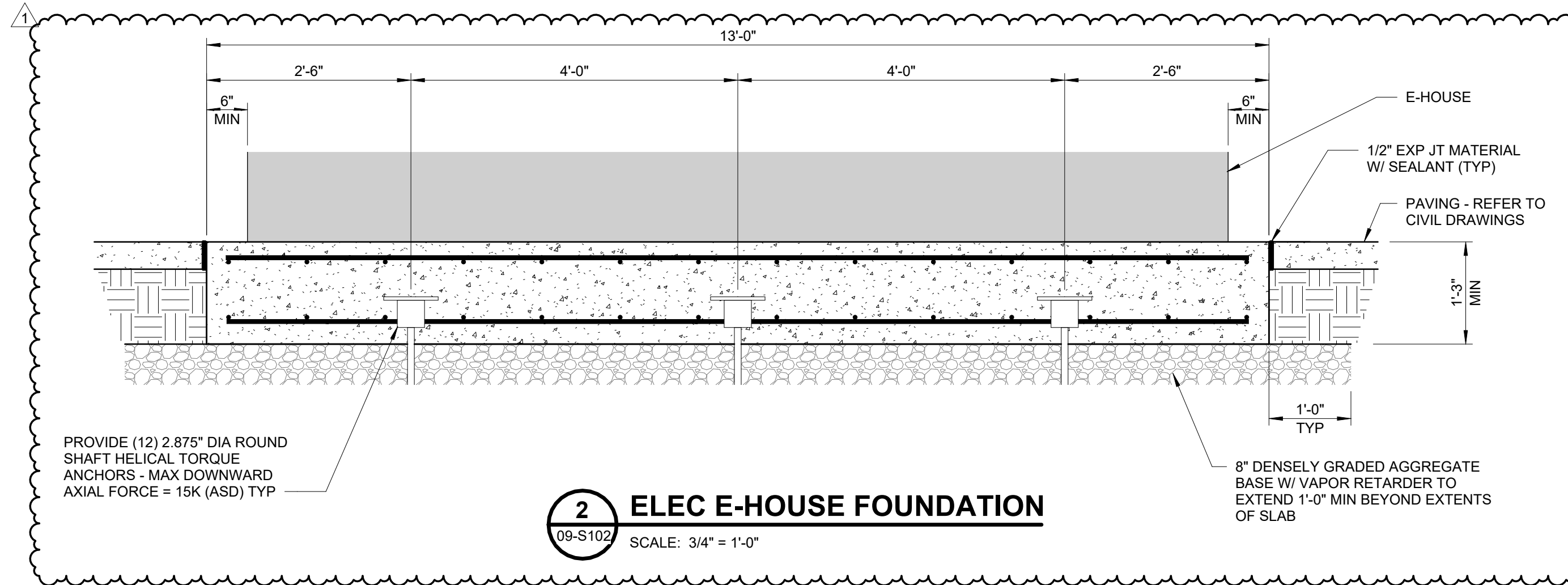
Table with columns: REV, CHK, DATE, DESCRIPTION, ISSUED FOR/BID, ADDENDUM

09-S001
FILE NO.: 3618121

Drawing Set: E-3  
 Drawing: ELECTRICAL E-HOUSE FOUNDATION PLAN  
 Title: ELECTRICAL E-HOUSE FOUNDATION PLAN  
 Date: 8/14/24



1  
 09-S102 SCALE: 1/8" = 1'-0"  
**ELECTRICAL E-HOUSE - FOUNDATION PLAN**



**PLAN NOTES**

- FOR GENERAL NOTES SEE SHEET 09-S001
- COORDINATE LOCATION AND SIZE OF E-HOUSE FOUNDATIONS WITH ELECTRICAL AND PROCESS PRIOR TO CONSTRUCTION.
- ELECTRICAL E-HOUSE BUILDING STRUCTURE TO BE DESIGNED PER THE DESIGN CRITERIA LISTED ON SHEET 09-S001

**BARGE**  
 DESIGN SOLUTIONS  
 615 3rd Avenue, Suite 100, Macon, GA 31201  
 Phone: 615.254.1200 / Fax: 615.254.6772



**ELECTRICAL E-HOUSE - FOUNDATION PLAN**

**LOWER POPLAR WATER RECLAMATION FACILITY  
 INFLUENT PUMP STATION IMPROVEMENTS  
 MACON WATER AUTHORITY**

REVISION INFORMATION		DESCRIPTION
REV.	CHK.	DATE
0	JBA	07/10/2024
1	JBA	08/14/2024

**09-S102**  
 FILE NO.: 3618121

**PLAN NOTES**

- FOR GENERAL NOTES SEE SHEET 09-S001
- 4" THICK LW CONCRETE TOPPING SLAB SHALL ADHERE TO THE FOLLOWING SPECIFICATIONS
  - 4,500PSI COMPRESSIVE STRENGTH W/ CTS TYPE K CEMENT
  - FORTA MACRO-SYNTHETIC FIBERS (7.5 LB/CY DOSAGE)
- COORDINATE SIZE AND LOCATIONS OF HATCHES AND OPENINGS PRIOR TO DEMOLITION OF CONCRETE OR FABRICATION OF STEEL.
- ALL T/STL = 294.34' UNO.
- INDICATES AREAS OF CONCRETE INFILL TO BE POURED MONOLITHICALLY WITH TOPPING SLAB. SEE DET 10/09-S301 FOR REINFORCING.
- SEE SHEET 09-S501 FOR PILE CAP SECTIONS AND DETAILS.
- TIGHTLY PACK 1" NON-SHRINK GROUT BETWEEN TOP OF BEAM AND BOTTOM OF SLAB.
- ALL NEW BEAMS TO BE STAINLESS STEEL.
- CONTRACTOR TO LOCATE ALL UNDERGROUND UTILITIES AND POTENTIAL INTERFERENCES PRIOR TO INSTALLING HELICAL ANCHORS.
- EXISTING OPENING - CLEAN AS REQ'D TO WELD EXISTING LID CLOSED PRIOR TO POURING TOPPING SLAB.

**BARGE**  
DESIGN SOLUTIONS

615 3rd Avenue, Suite 1000, Atlanta, GA 30308, Telephone: 404.525.3710  
Fax: 404.525.6072

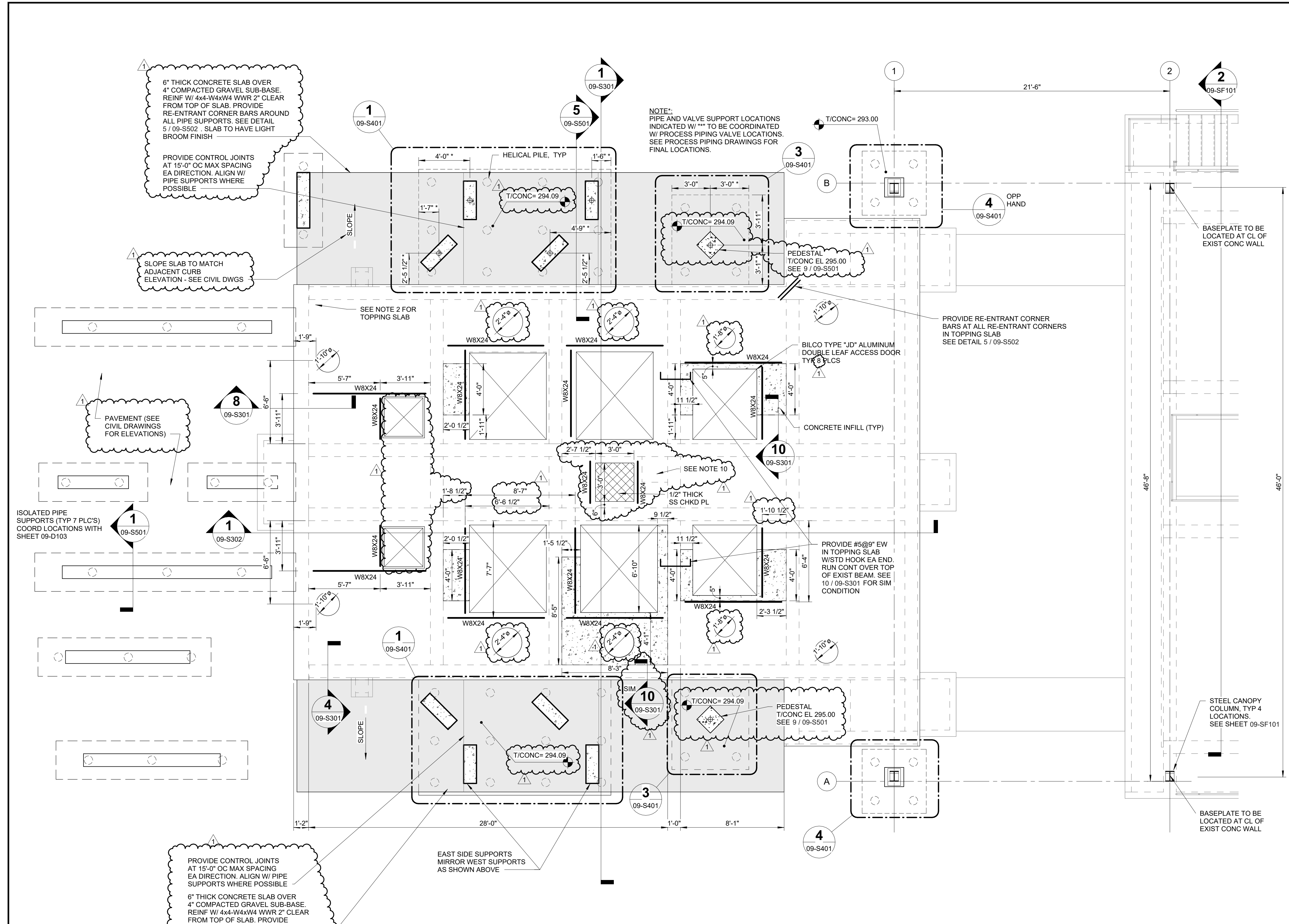


**PUMP STATION - UPPER LEVEL PLAN**  
LOWER POPLAR WATER RECLAMATION FACILITY  
INFLUENT PUMP STATION IMPROVEMENTS  
MACON WATER AUTHORITY

REVISION INFORMATION		DESCRIPTION
REV.	CHK.	DATE
0	JBA	07/10/2024
1	JBA	08/14/2024

**09-S103**

FILE NO.: 3618121



**1** **PUMP STATION - UPPER PLAN (ELEV 294.92)**

SCALE: 1/4" = 1'-0"

TRUE NORTH

6" THICK CONCRETE SLAB OVER 4" COMPACTED GRAVEL SUB-BASE. REINF W/ 4x4-W4xW4 WWR 2" CLEAR FROM TOP OF SLAB. PROVIDE RE-ENTRANT CORNER BARS AROUND ALL PIPE SUPPORTS. SEE DETAIL 5 / 09-S502. SLAB TO HAVE LIGHT BROOM FINISH

PROVIDE CONTROL JOINTS AT 15'-0" OC MAX SPACING EA DIRECTION. ALIGN W/ PIPE SUPPORTS WHERE POSSIBLE

6" THICK CONCRETE SLAB OVER 4" COMPACTED GRAVEL SUB-BASE. REINF W/ 4x4-W4xW4 WWR 2" CLEAR FROM TOP OF SLAB. PROVIDE RE-ENTRANT CORNER BARS AROUND ALL PIPE SUPPORTS. SEE DETAIL 5 / 09-S502. SLAB TO HAVE LIGHT BROOM FINISH

PROVIDE CONTROL JOINTS AT 15'-0" OC MAX SPACING EA DIRECTION. ALIGN W/ PIPE SUPPORTS WHERE POSSIBLE

NOTE: PIPE AND VALVE SUPPORT LOCATIONS INDICATED W/ "" TO BE COORDINATED W/ PROCESS PIPING VALVE LOCATIONS. SEE PROCESS PIPING DRAWINGS FOR FINAL LOCATIONS.

PEDESTAL T/CONC EL 295.00 SEE 9 / 09-S501

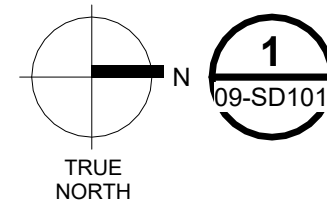
PROVIDE #5@9" EW IN TOPPING SLAB W/STD HOOK EA END. RUN CONT OVER TOP OF EXIST BEAM SEE 10 / 09-S301 FOR SIM CONDITION

BILCO TYPE "JD" ALUMINUM DOUBLE LEAF ACCESS DOOR TYR & PLCS

PEDESTAL T/CONC EL 295.00 SEE 9 / 09-S501

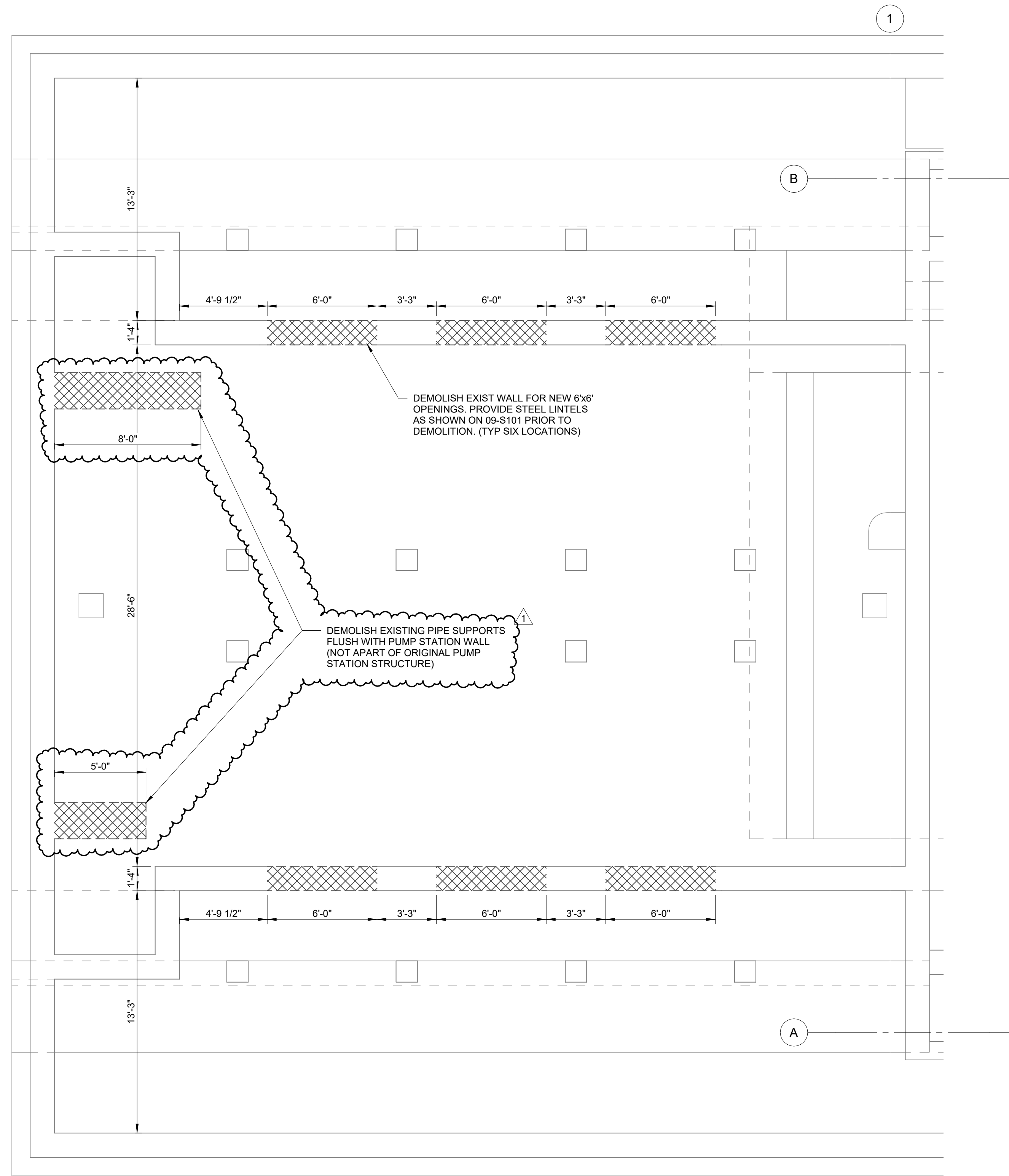
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 Drawing: 09-S103  
 Title: PUMP STATION IMPROVEMENTS  
 Date: 8/14/2024 2:01:05 PM  
 User: STRELL

Drawing Set: 4.1  
 Drawing: 09-SD101-LOWER DEMOLITION PLAN  
 Title: 09-SD101-LOWER DEMOLITION PLAN  
 Date: 08/14/24 2:02:34 PM



**1** **PUMP STATION - LOWER DEMOLITION PLAN (ELEV 260.58)**

09-SD101 SCALE: 1/4" = 1'-0"



**PLAN NOTES**

1. REF SHEET 09-S103. BEAMS MUST BE INSTALLED AND IN PLACE PRIOR TO SLAB DEMOLITION.
2. EXISTING HOUSEKEEPING PADS TO BE DEMOLISHED DOWN FLUSH WITH EXISTING TOP OF SLAB ELEVATION.
3. WHERE SAWCUTTING CONCRETE EXPOSES REBAR, CLEAN CONCRETE AND REBAR SURFACE IN ACCORDANCE WITH SIKAWRITTEN REQUIREMENTS FOR INSTALLATION OF SIKAGARD-62. INSTALL TWO COATS OF SIKAGARD-62 (GRAY) OVER EXPOSED REBAR EXTENDING 3" MINIMUM PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
4. EXISTING STRUCTURAL DRAWINGS WILL BE MADE AVAILABLE TO CONTRACTOR UPON REQUEST.
5. SUBMIT A DETAILED DEMOLITION PLAN FOR REVIEW BEFORE BEGINNING DEMOLITION. COORDINATE ALL OPENING SIZES WITH APPROVED HATCH SHOP DRAWINGS.

**BARGE**  
 DESIGN SOLUTIONS  
 615 305 Avenue South, Suite 200 / Macon, GA, Tennessee 37210  
 Phone: 615.324.1420 / Fax: 615.251.6172



**PUMP STATION - LOWER DEMOLITION PLAN**

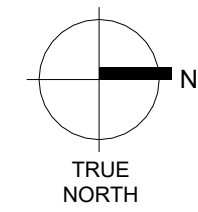
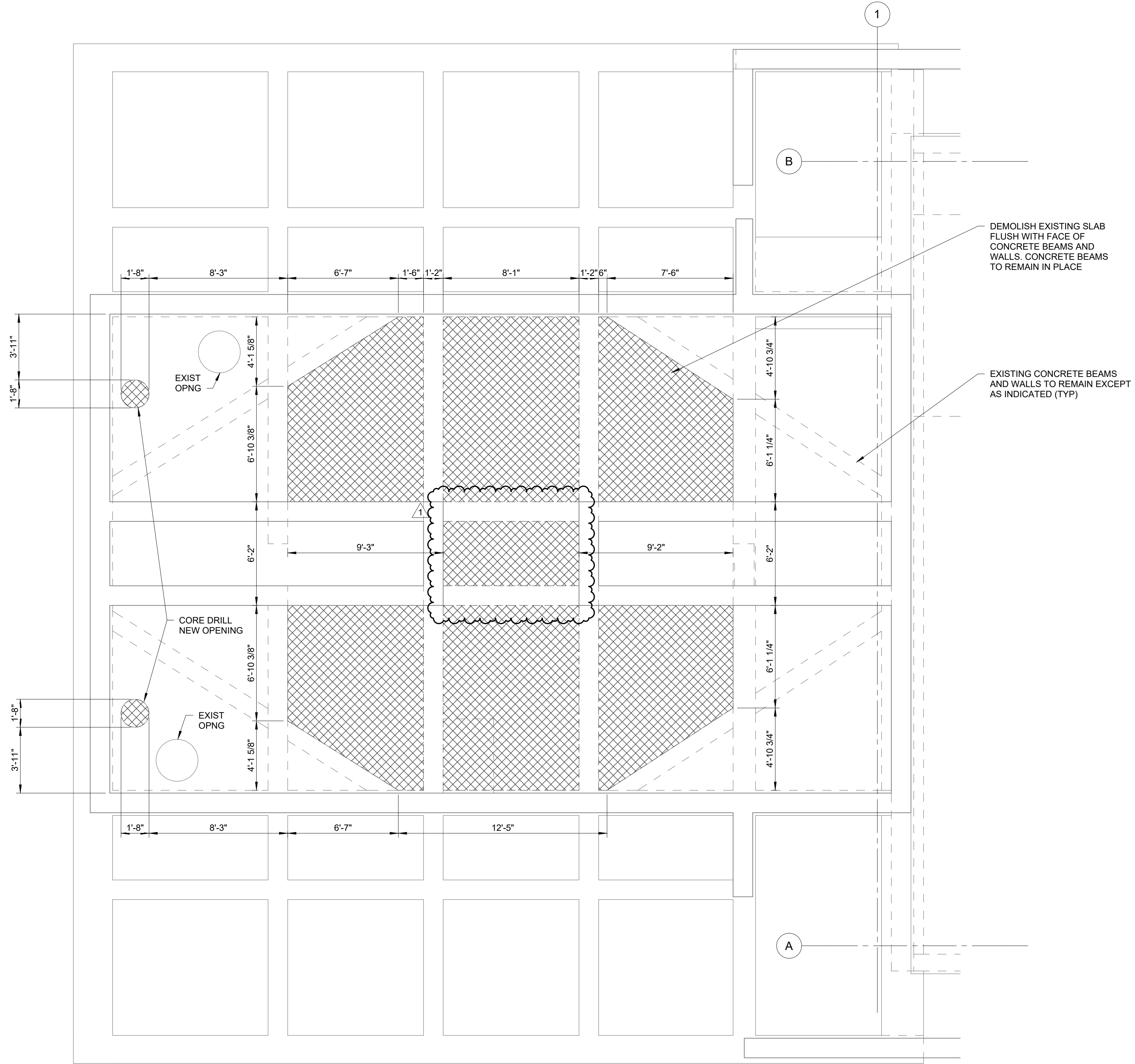
LOWER POPLAR WATER RECLAMATION FACILITY  
 INFLUENT PUMP STATION IMPROVEMENTS  
 MACON WATER AUTHORITY

REV.	DR.	CHK.	DATE	DESCRIPTION
0	ACM	JBA	07/10/2024	ISSUED FOR BID
1	MRD	JBA	08/14/2024	ADDENDUM 3

**09-SD101**

FILE NO.: 3618121

Drawing Set: 4.1  
 Drawing: 09-SD102  
 Title: PUMP STATION - INTERMEDIATE DEMOLITION PLAN  
 Date: 8/14/24  
 File Path: \\server\projects\09-SD102\09-SD102.dwg

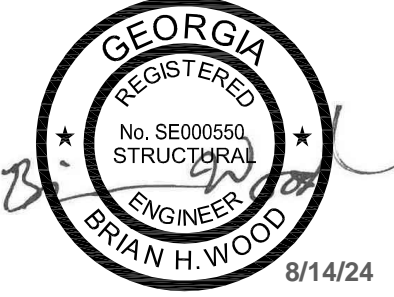


**1 PUMP STATION - INTERMEDIATE DEMOLITION PLAN (ELEV 273.75)**

09-SD102 SCALE: 1/4" = 1'-0"

**PLAN NOTES**

1. REF SHEET 09-S103. BEAMS MUST BE INSTALLED AND IN PLACE PRIOR TO SLAB DEMOLITION.
2. EXISTING HOUSEKEEPING PADS TO BE DEMOLISHED DOWN FLUSH WITH EXISTING TOP OF SLAB ELEVATION.
3. WHERE SAWCUTTING CONCRETE EXPOSES REBAR, CLEAN CONCRETE AND REBAR SURFACE IN ACCORDANCE WITH SIKAWRITTEN REQUIREMENTS FOR INSTALLATION OF SIKAGARD-62. INSTALL TWO COATS OF SIKAGARD-62 (GRAY) OVER EXPOSED REBAR EXTENDING 3" MINIMUM PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
4. EXISTING STRUCTURAL DRAWINGS WILL BE MADE AVAILABLE TO CONTRACTOR UPON REQUEST.
5. SUBMIT A DETAILED DEMOLITION PLAN FOR REVIEW BEFORE BEGINNING DEMOLITION. COORDINATE ALL OPENING SIZES WITH APPROVED HATCH SHOP DRAWINGS.



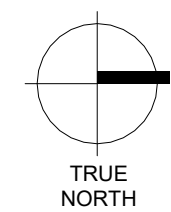
**BARGE**  
 DESIGN SOLUTIONS  
 615 3rd Avenue, Suite 1000, Macon, GA 31201  
 Phone: 615.324.1420 / Fax: 615.324.5672

**PUMP STATION - INTERMEDIATE  
 DEMOLITION PLAN**  
 LOWER POPLAR WATER RECLAMATION FACILITY  
 INFLUENT PUMP STATION IMPROVEMENTS  
 MACON WATER AUTHORITY

REV.	DR.	CHK.	DATE	DESCRIPTION
0	ACM	JBA	07/10/2024	ISSUED FOR BID
1	MRD	JBA	08/14/2024	ADDENDUM 3

**09-SD102**  
FILE NO.: 3618121

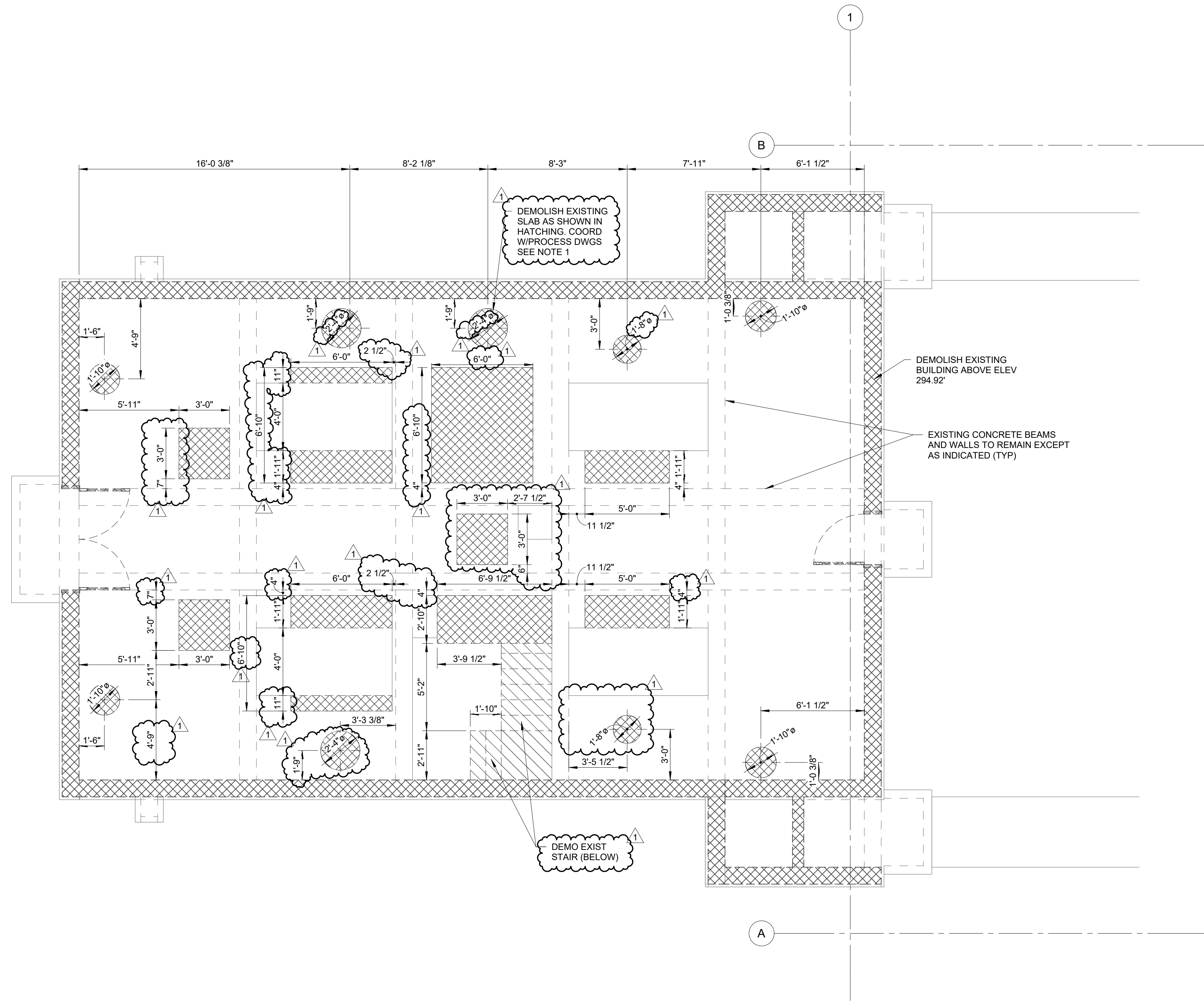
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 Title: UPPER DEMOLITION PLAN  
 Date: 8/14/24



**1**  
09-SD103

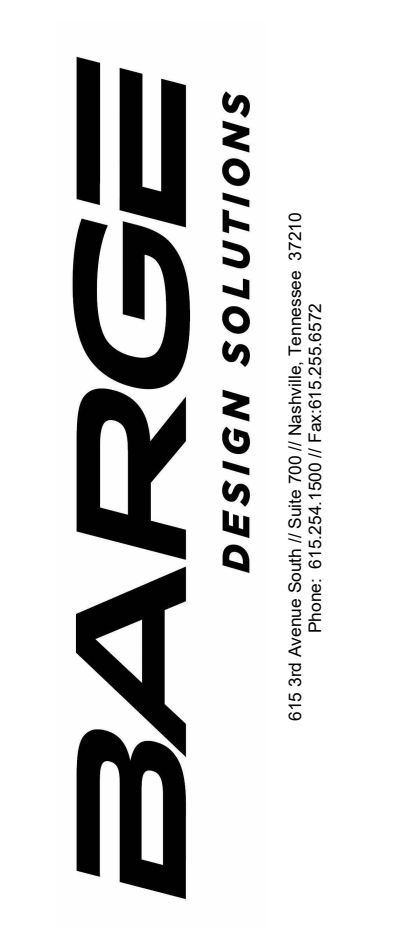
**PUMP STATION - UPPER DEMOLITION PLAN (ELEV 294.92)**

SCALE: 1/4" = 1'-0"



**PLAN NOTES**

1. REF SHEET 09-S103. BEAMS MUST BE INSTALLED AND IN PLACE PRIOR TO SLAB DEMOLITION.
2. EXISTING HOUSEKEEPING PADS TO BE DEMOLISHED DOWN FLUSH WITH EXISTING TOP OF SLAB ELEVATION.
3. WHERE SAWCUTTING CONCRETE EXPOSES REBAR, CLEAN CONCRETE AND REBAR SURFACE IN ACCORDANCE WITH SIKA WRITTEN REQUIREMENTS FOR INSTALLATION OF SIKAGARD-62. INSTALL TWO COATS OF SIKAGARD-62 (GRAY) OVER EXPOSED REBAR EXTENDING 3" MINIMUM PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
4. EXISTING STRUCTURAL DRAWINGS WILL BE MADE AVAILABLE TO CONTRACTOR UPON REQUEST.
5. SUBMIT A DETAILED DEMOLITION PLAN FOR REVIEW BEFORE BEGINNING DEMOLITION. COORDINATE ALL OPENING SIZES WITH APPROVED HATCH SHOP DRAWINGS.



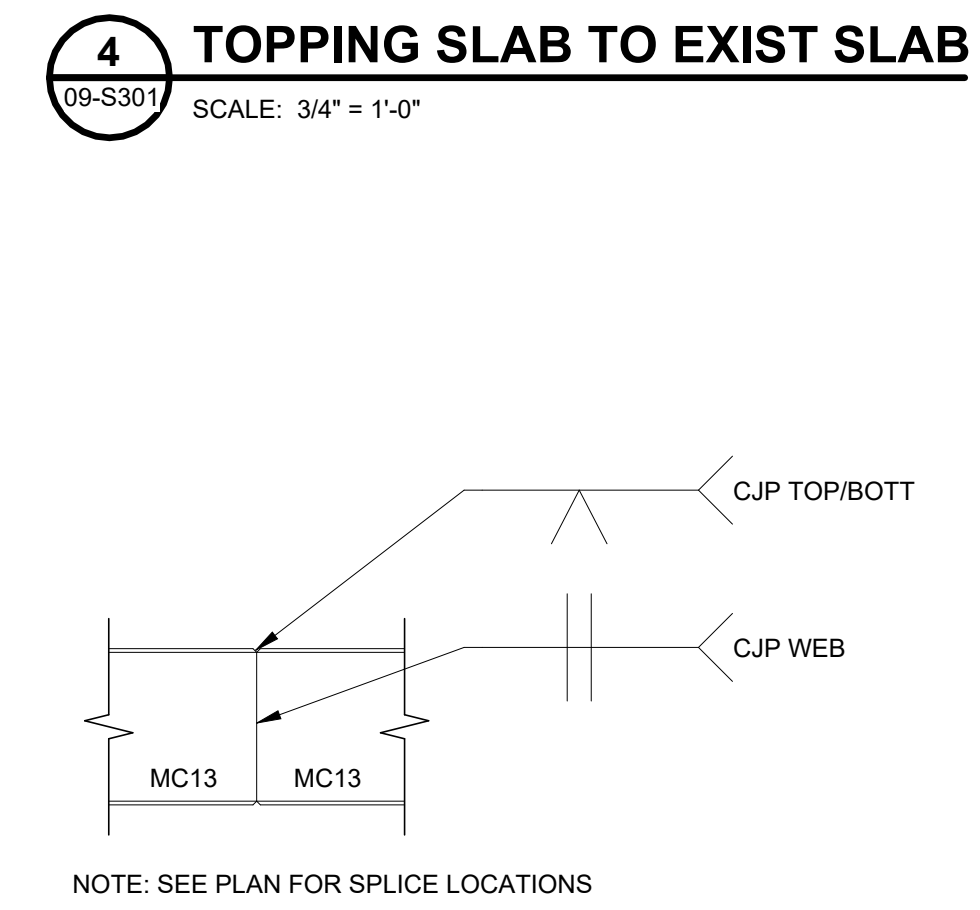
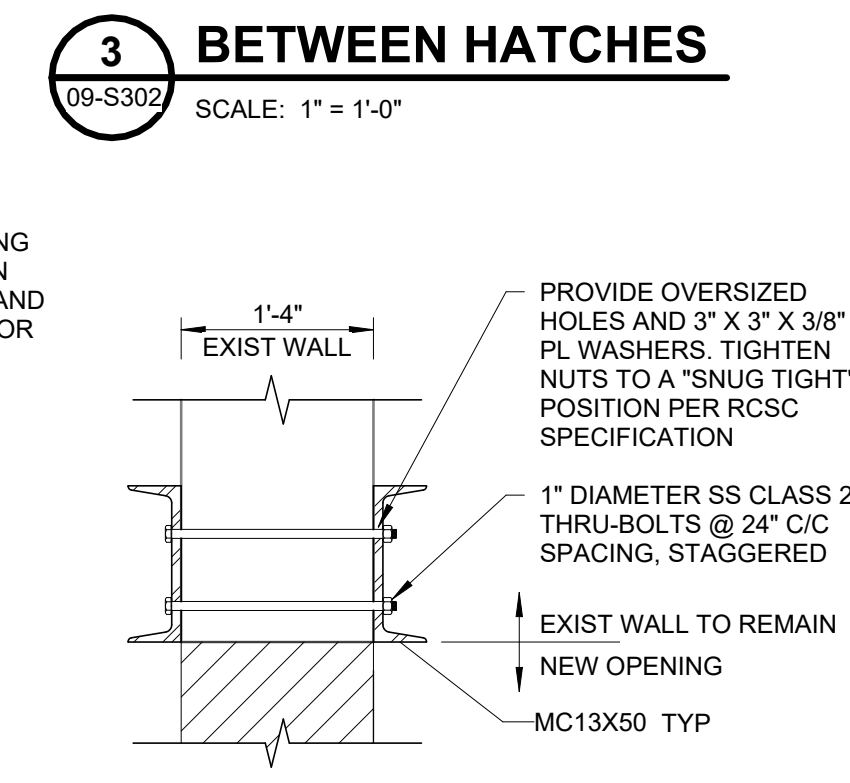
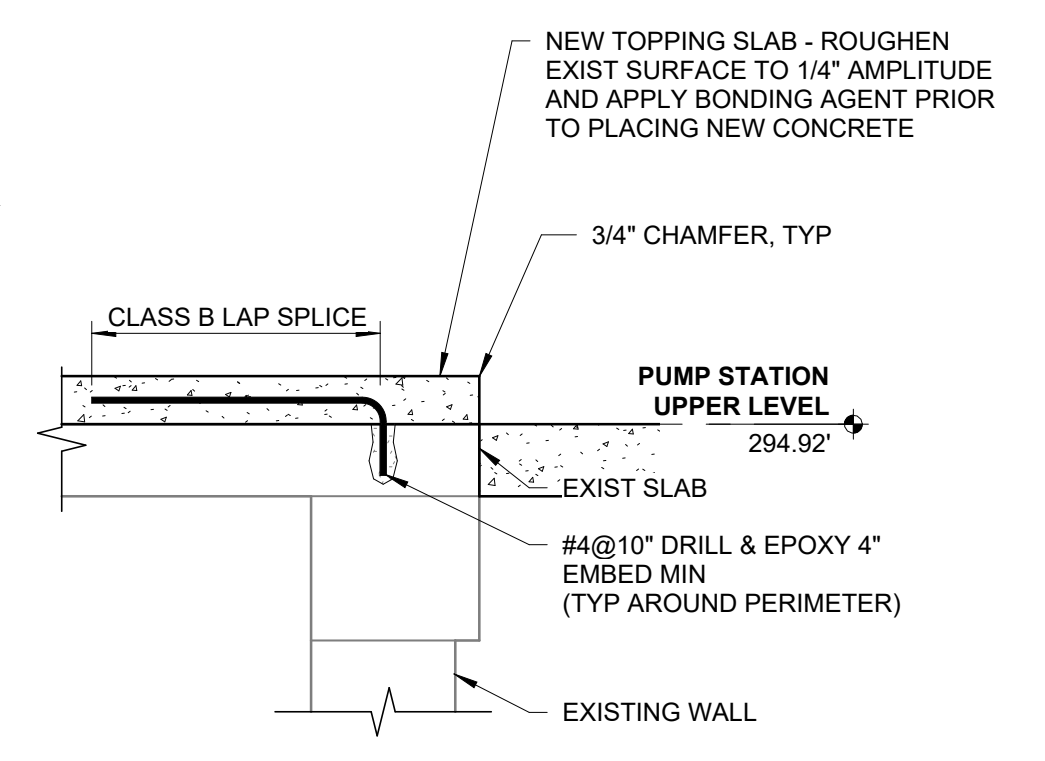
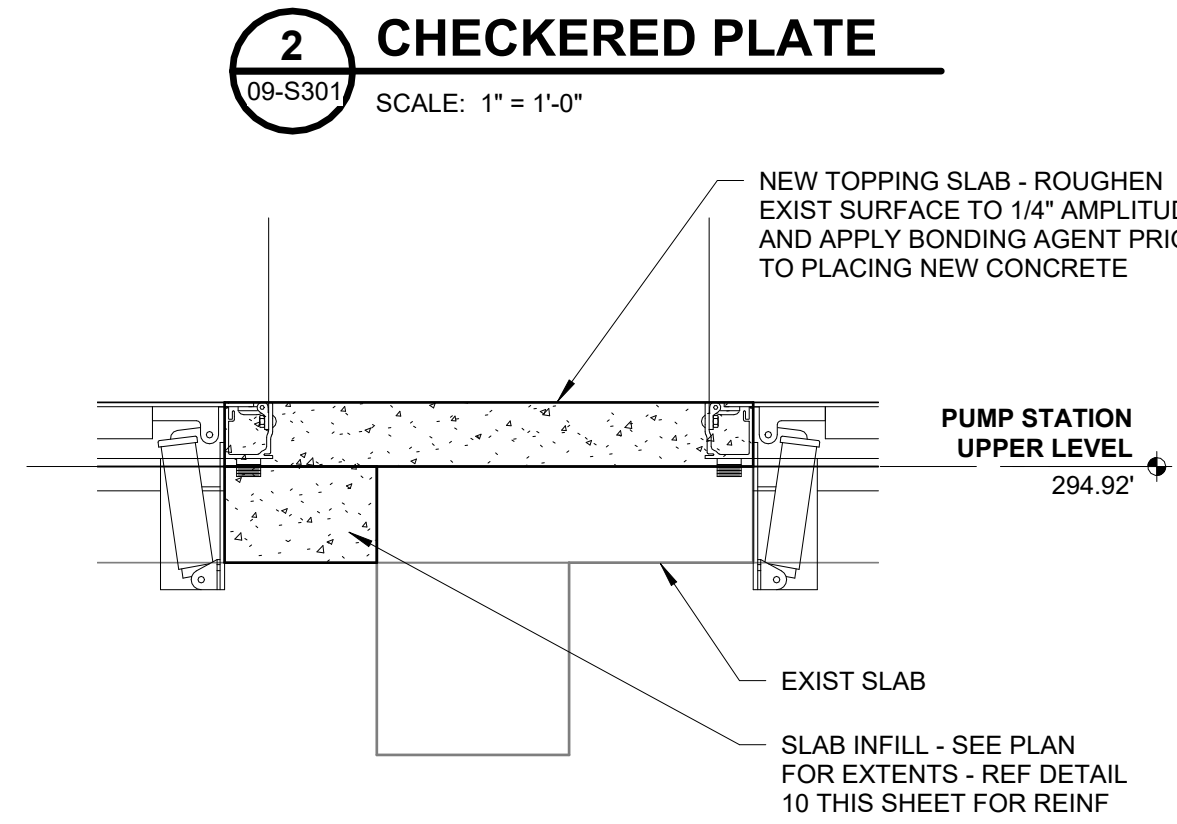
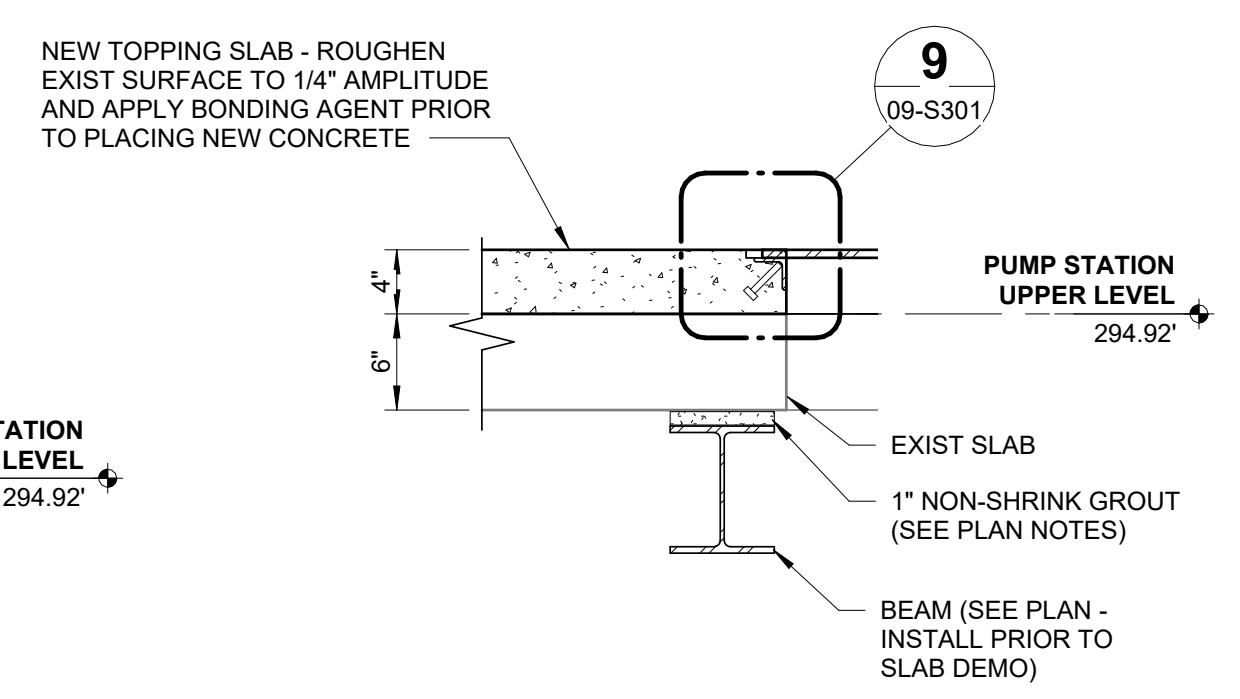
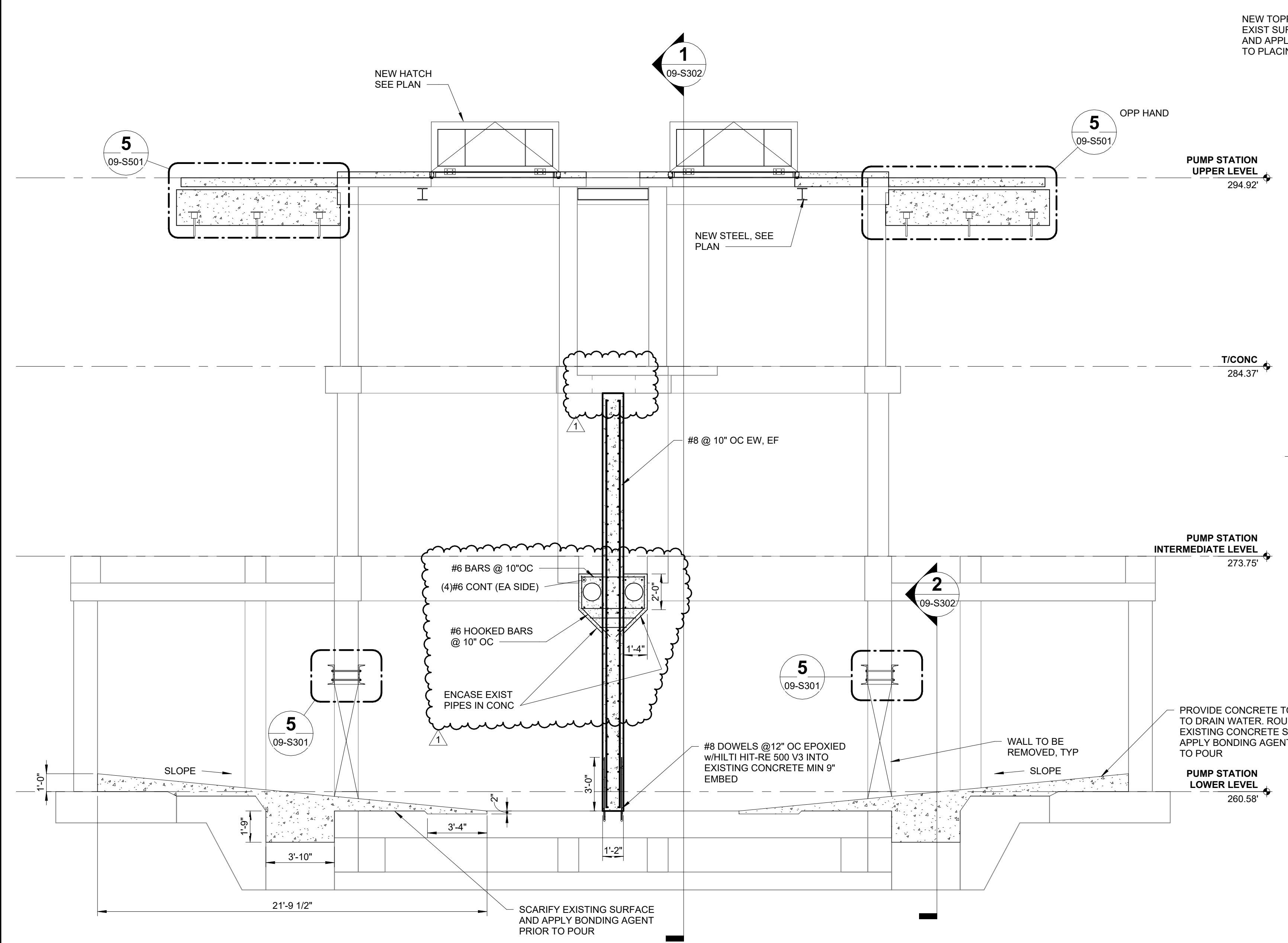
**PUMP STATION - UPPER DEMOLITION PLAN**  
 LOWER POPLAR WATER RECLAMATION FACILITY  
 INFLUENT PUMP STATION IMPROVEMENTS  
 MACON WATER AUTHORITY

REVISION INFORMATION		DESCRIPTION
REV.	CHK.	DATE
0	JBA	07/10/2024
1	JBA	08/14/2024

**09-SD103**  
FILE NO.: 3618121



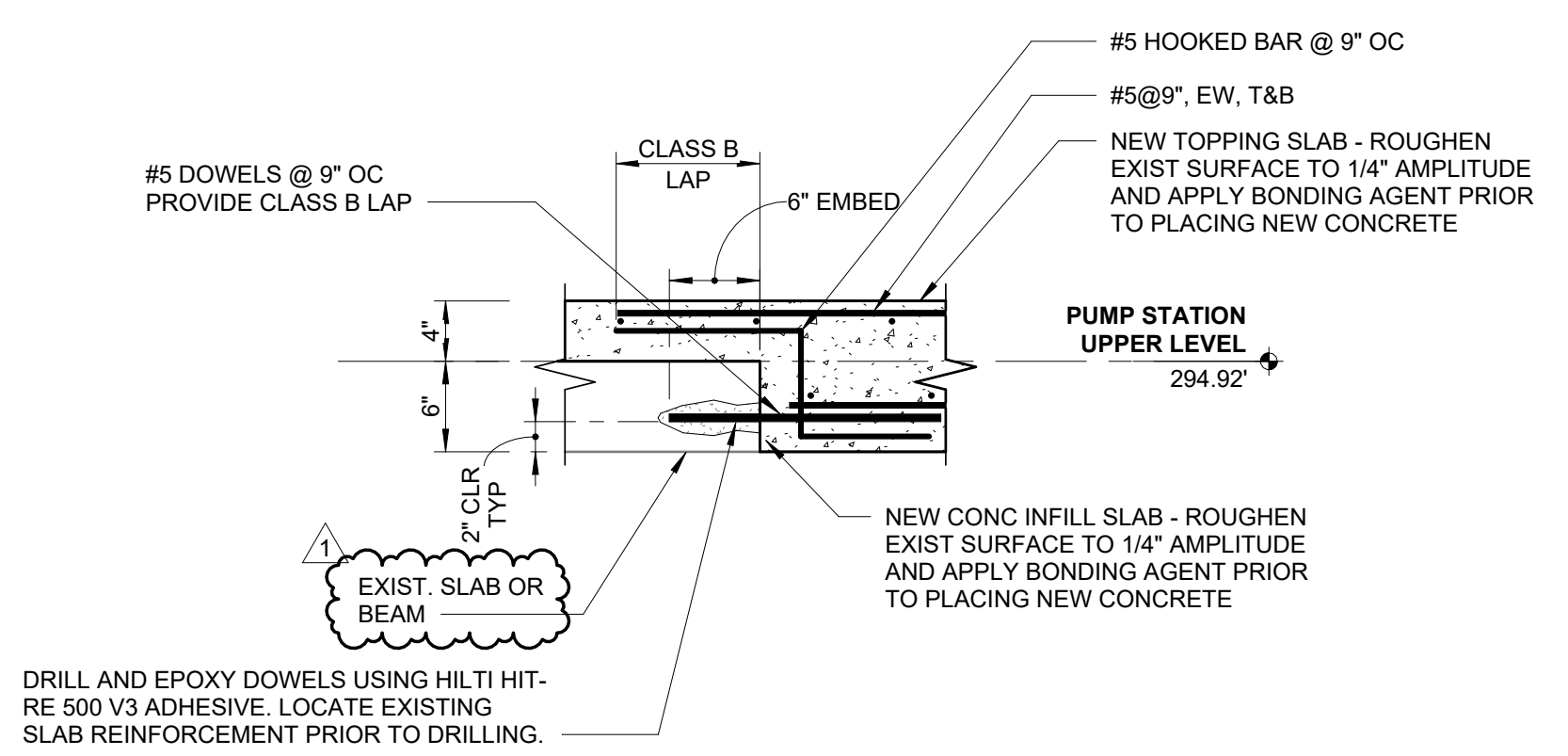
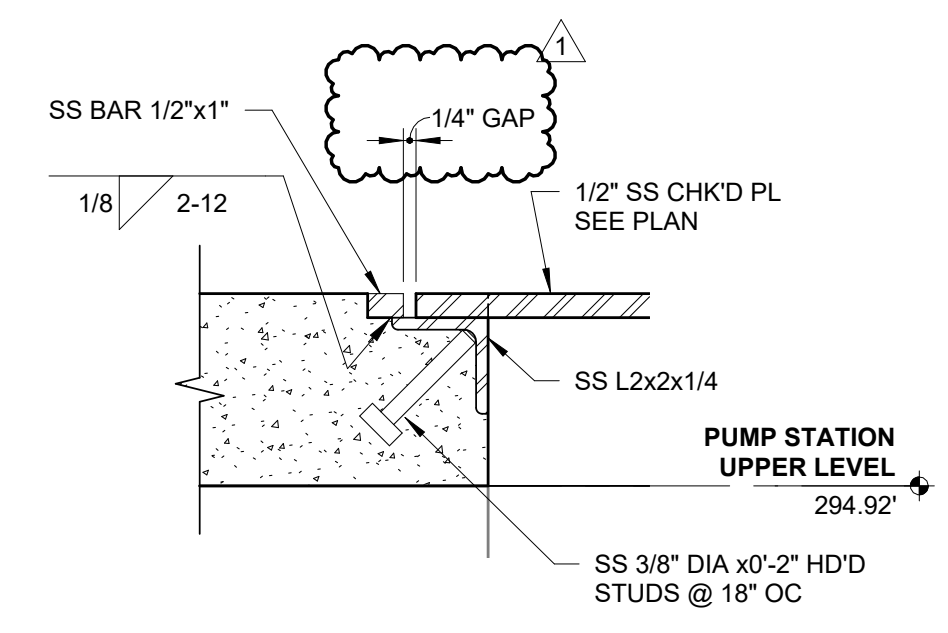
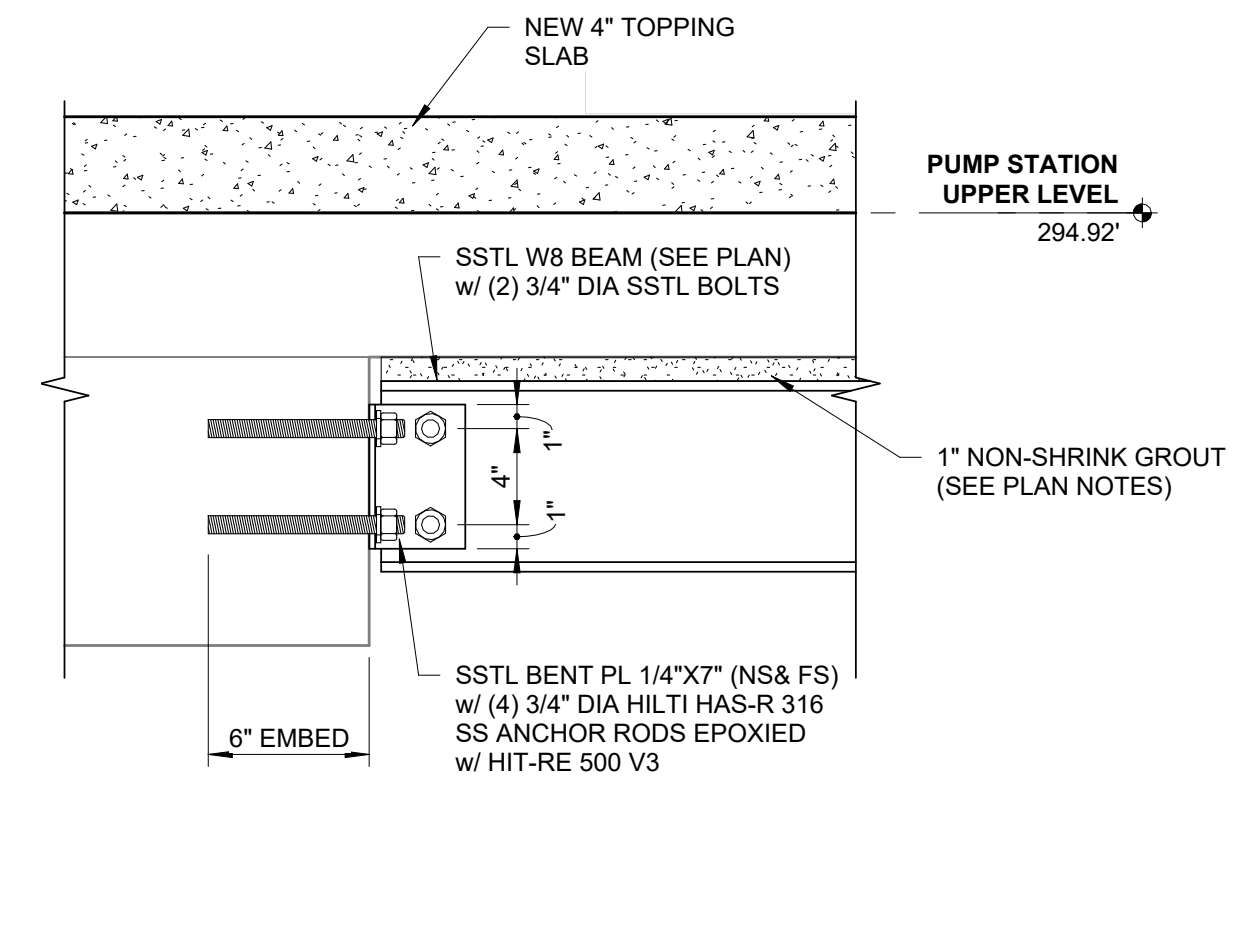
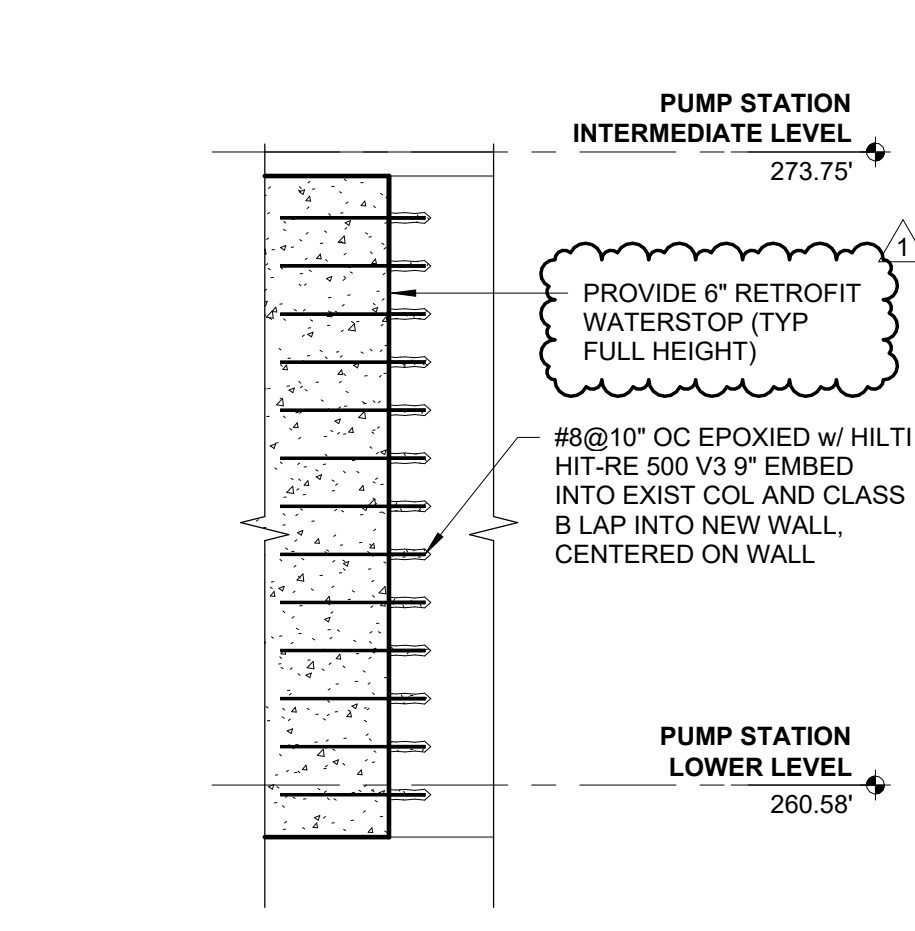
SECTIONS  
 LOWER POPLAR WATER RECLAMATION FACILITY  
 INFLUENT PUMP STATION IMPROVEMENTS  
 MACON WATER AUTHORITY



**1 SECTION**  
 09-S101 SCALE: 1/4" = 1'-0"

**5 STEEL LINTEL**  
 09-S301 SCALE: 3/4" = 1'-0"

**6 CHANNEL SPLICE**  
 09-S301 SCALE: 3/4" = 1'-0"



**7 EXISTING COL INTERFACE**  
 09-S301 SCALE: 1/4" = 1'-0"

**8 BEAM TO CONCRETE**  
 09-S301 SCALE: 1 1/2" = 1'-0"

**9 ARMORED EDGE**  
 09-S301 SCALE: 3" = 1'-0"

**10 CONCRETE INFILL**  
 09-S103 SCALE: 1" = 1'-0"

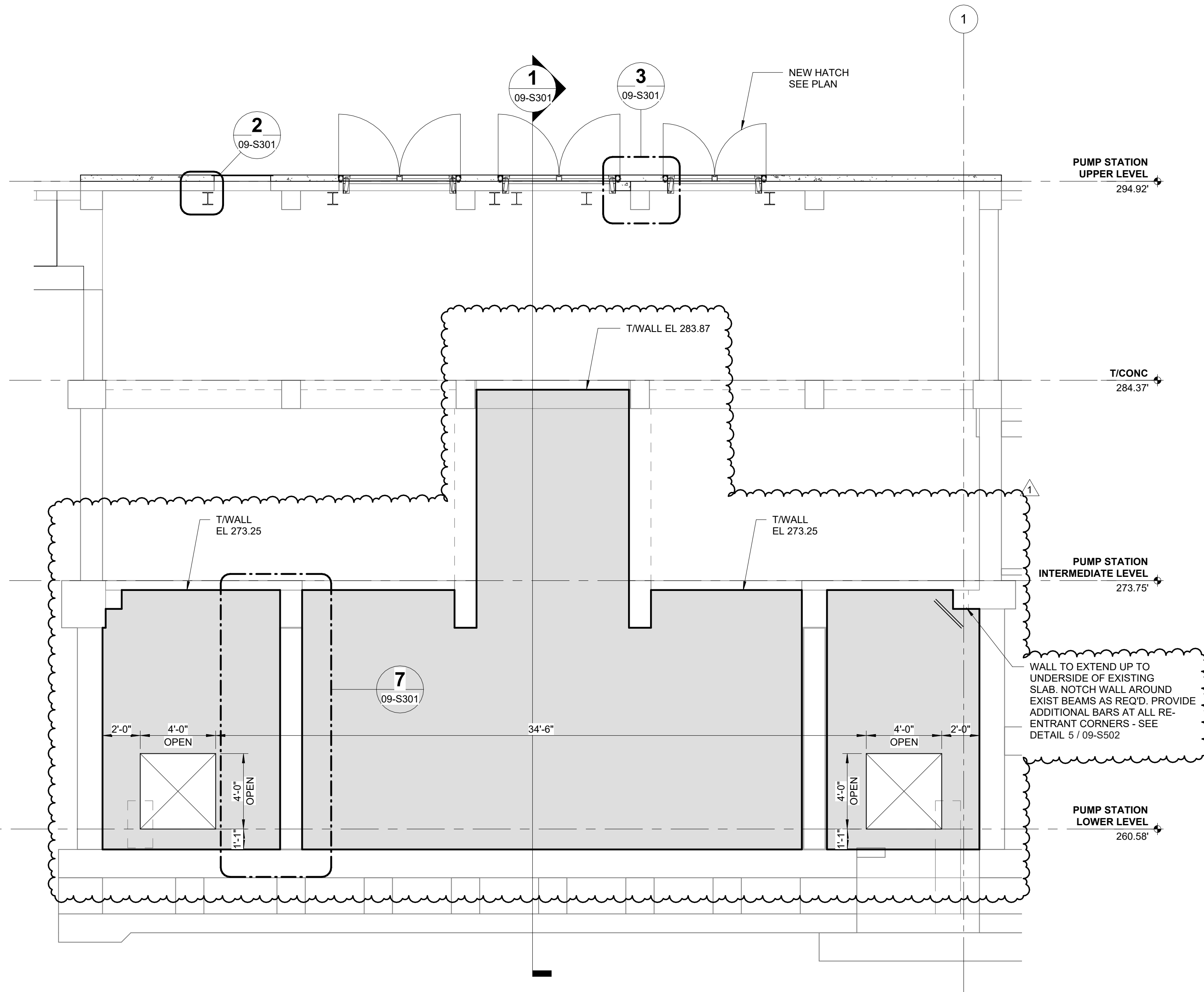
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 Drawing: 09-S301  
 Title: Lower Poplar Water Reclamation Facility  
 Date: 8/14/24  
 File: 09-S301.dwg

REVISION INFORMATION		DESCRIPTION
CHK	JBA	ISSUED FOR BID
DR	JBA	08/14/2024
ACM	JBA	ADDENDUM 3
REV	0	
REV	1	

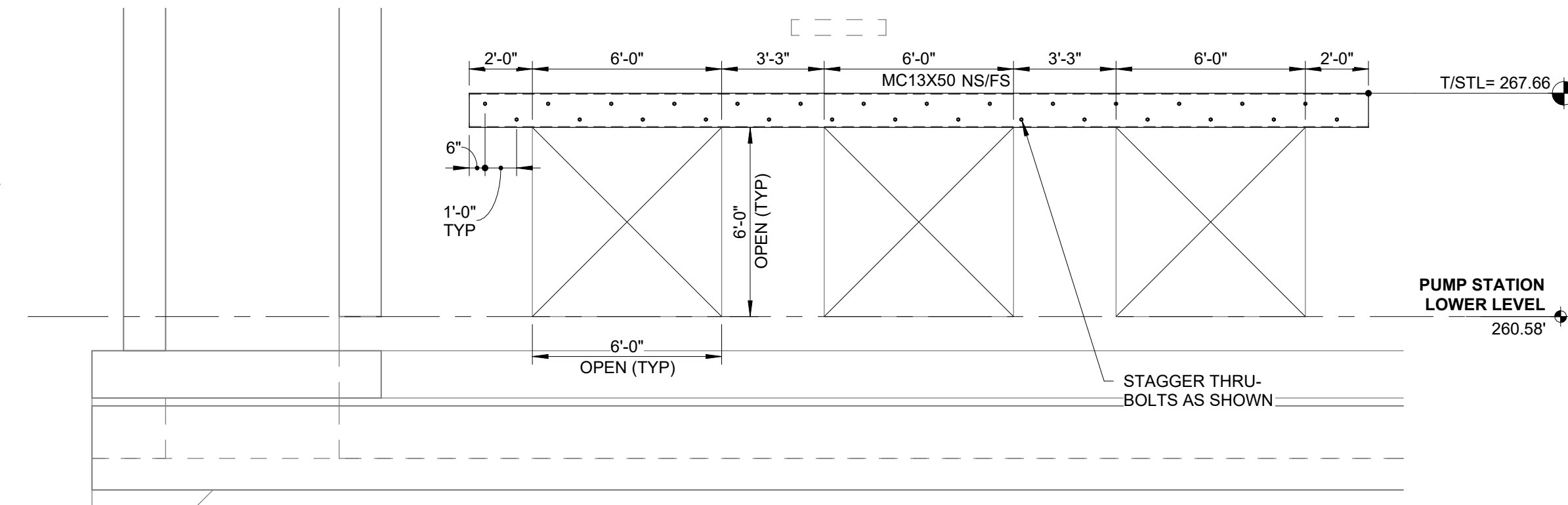
**09-S301**  
 FILE NO.: 3618121



Drawing Set: 09-S302  
 Drawing: 09-S302-1  
 Title: GATE OPENINGS  
 Date: 8/14/24



**1 GATE OPENINGS**  
 09-S302 SCALE: 1/4" = 1'-0"



**2 SHORING SECTION**  
 09-S302 SCALE: 1/4" = 1'-0"

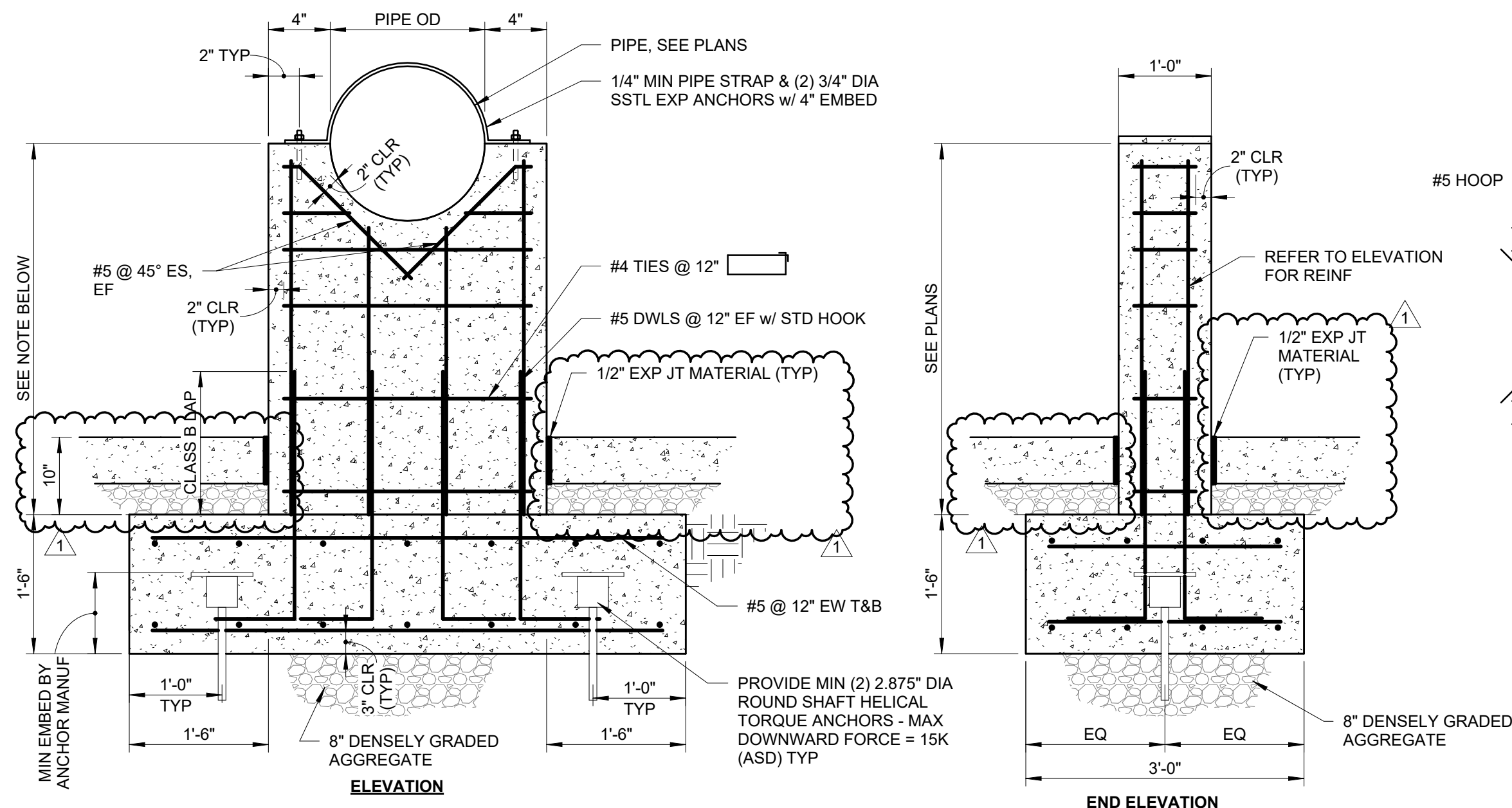


**SECTIONS**  
 LOWER POPLAR WATER RECLAMATION FACILITY  
 INFLUENT PUMP STATION IMPROVEMENTS  
 MACON WATER AUTHORITY

REV.	CHK.	DATE	DESCRIPTION
0	JBA	07/10/2024	ISSUED FOR BID
1	JBA	08/14/2024	ADDENDUM 3

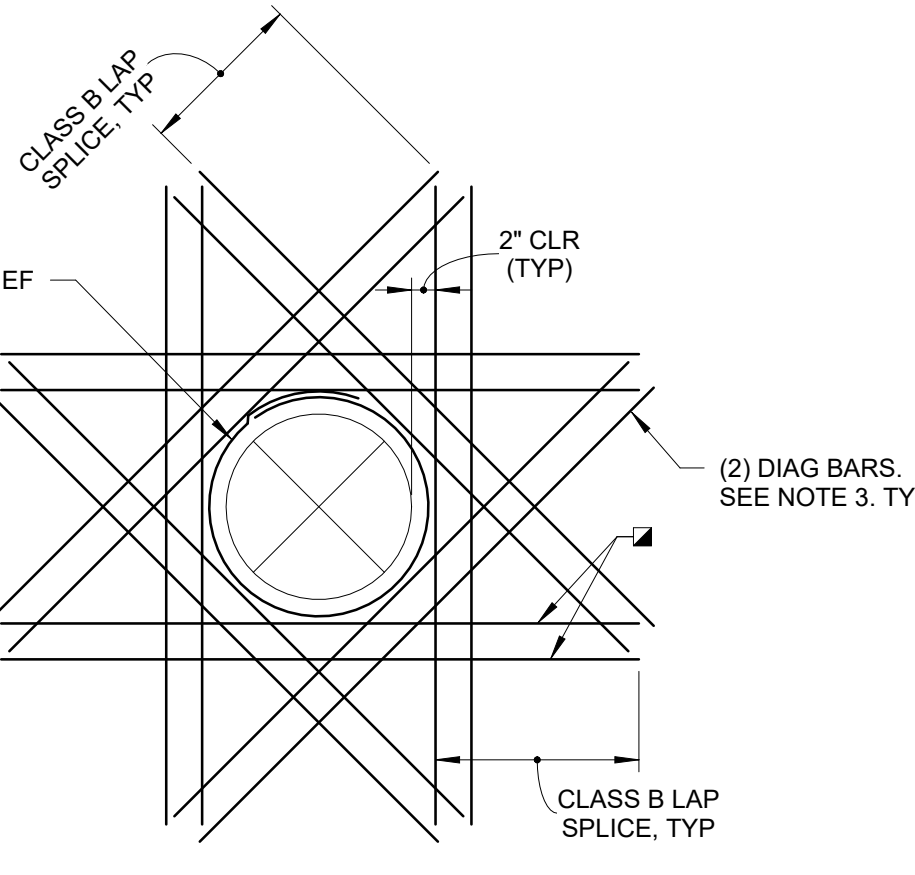
09-S302

FILE NO.: 3618121



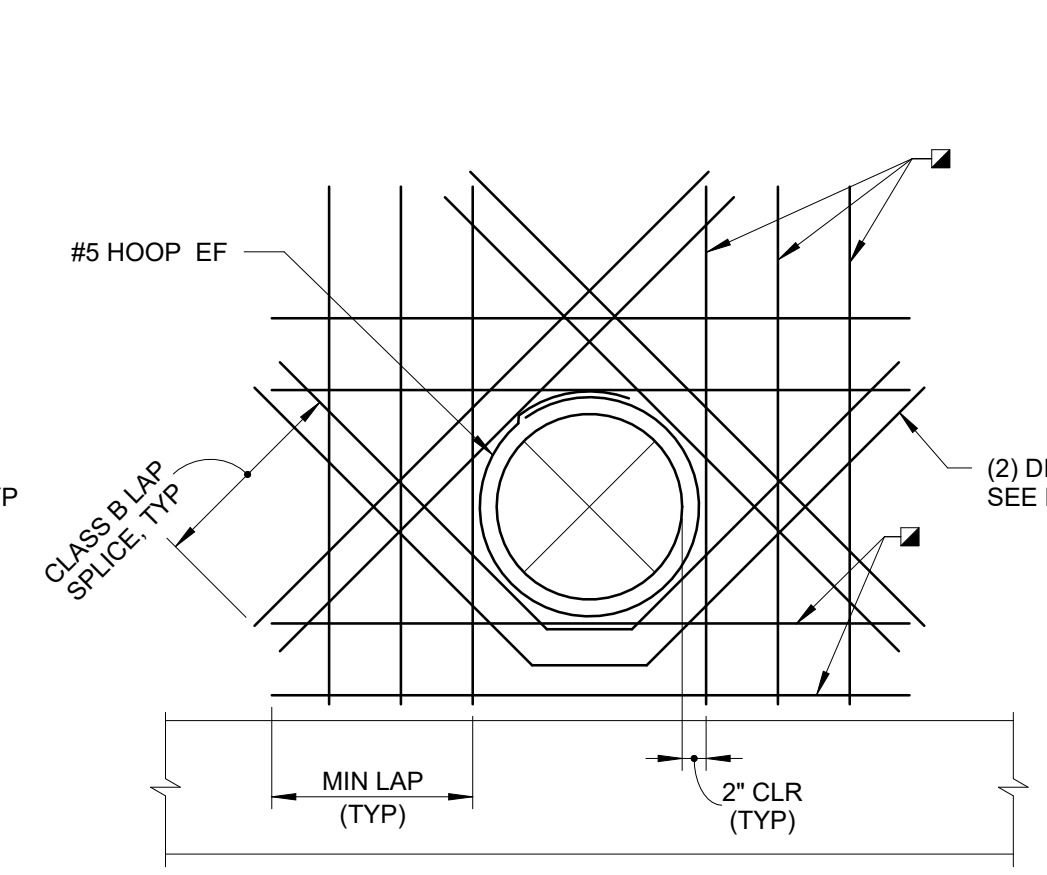
**1 PIPE SUPPORT**  
 09-S501 SCALE: 3/4" = 1'-0"

NOTE:  
 ALIGN TOP OF PEDESTAL ELEVATIONS W/ PIPE CENTERLINE. COORDINATE T/PEDESTAL W/ OTHER DISCIPLINE DRAWINGS. NOTE TYPICAL FOR ALL PIPE SUPPORT PEDESTALS.



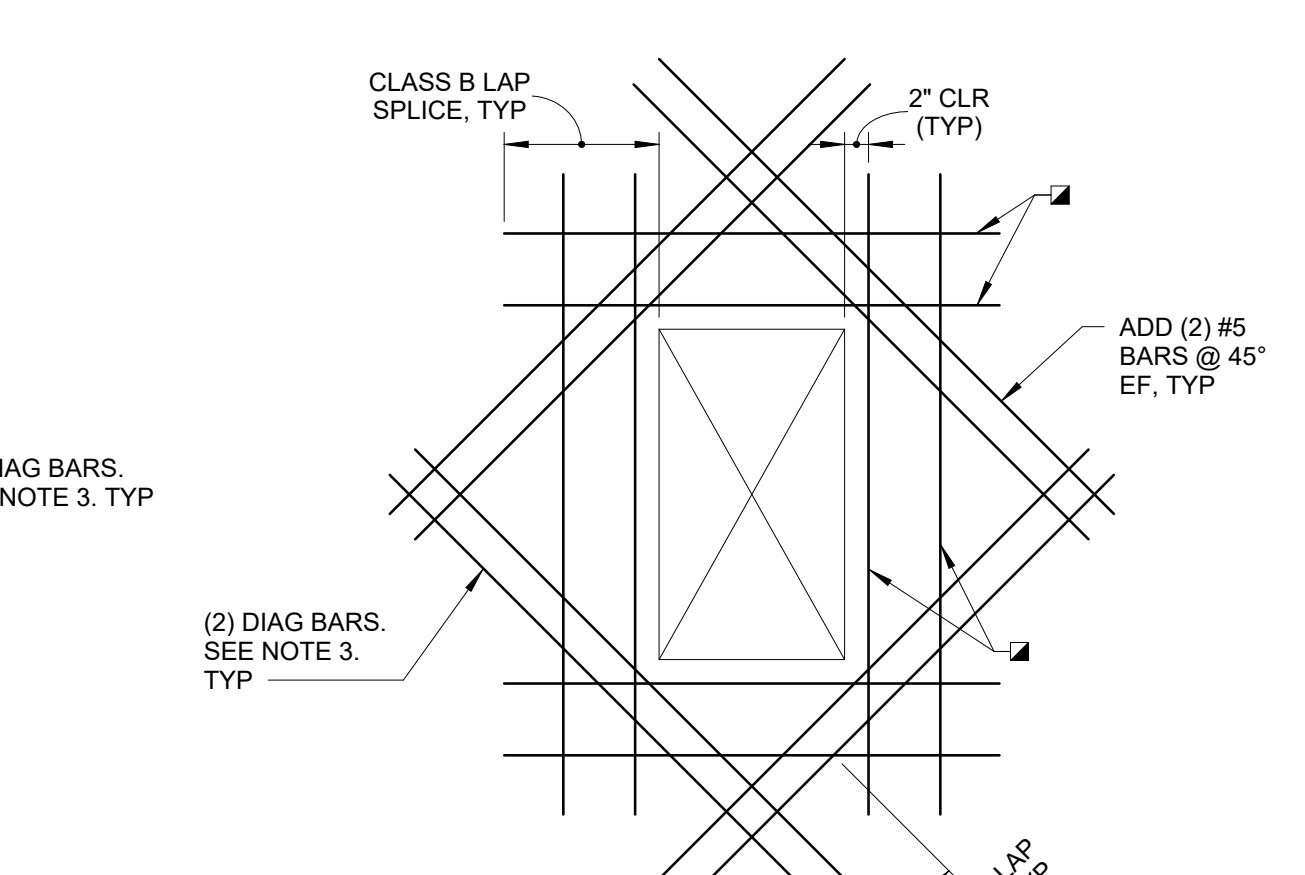
**2 ADDITIONAL REINFORCING AT OPENINGS**  
 09-S501 SCALE: 3/4" = 1'-0"

NOTES:  
 1. SIZE OF ADDITIONAL REINFORCING BARS TO EQUAL SIZE OF INTERRUPTED REINFORCING BARS.  
 2. PROVIDE STANDARD HOOKS FOR BARS IF LAP LENGTH EXTENSION CANNOT BE OBTAINED AT JOINTS OR OTHER OBSTRUCTIONS. PLACE ADDITIONAL BARS IN SAME PLANES AS INTERRUPTED REINFORCING.  
 3. UNLESS NOTED OTHERWISE, SIZE OF DIAGONAL BARS SHALL BE THE SAME SIZE AS THE INTERRUPTED NORMAL REINFORCING.  
 4. PLACE DIAGONAL BARS INSIDE NORMAL REINFORCING.  
 5. ON EACH FACE, REPLACE HORIZONTAL AND VERTICAL BARS INTERRUPTED BY OPENING WITH BARS OF EQUAL SIZE AND NUMBER. MINIMUM OF HALF THE INTERRUPTED BARS ON EACH SIDE. SPACE ADDED BARS AT 6" OC



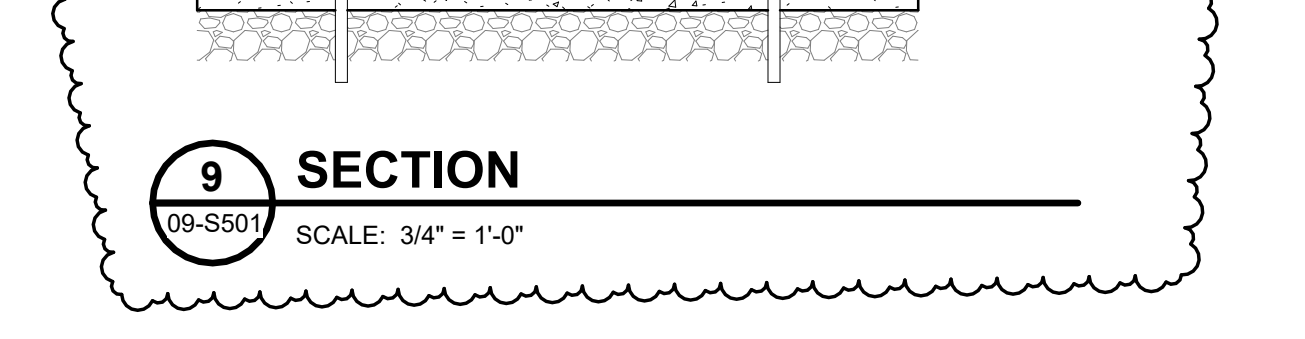
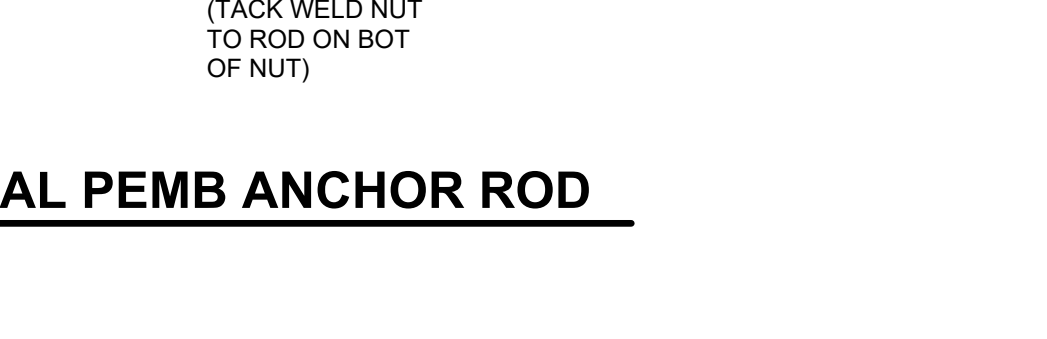
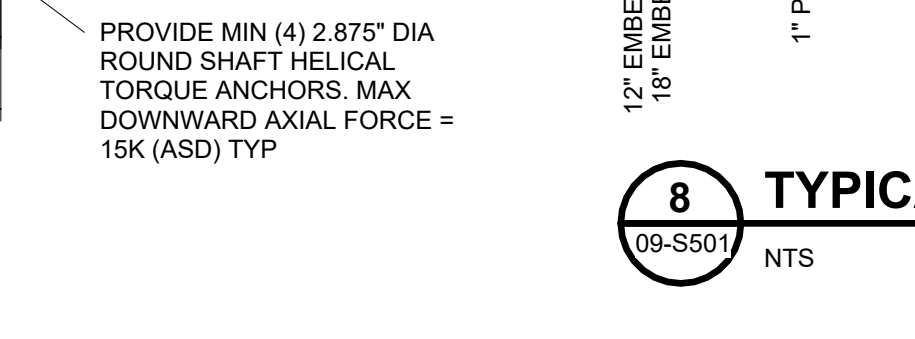
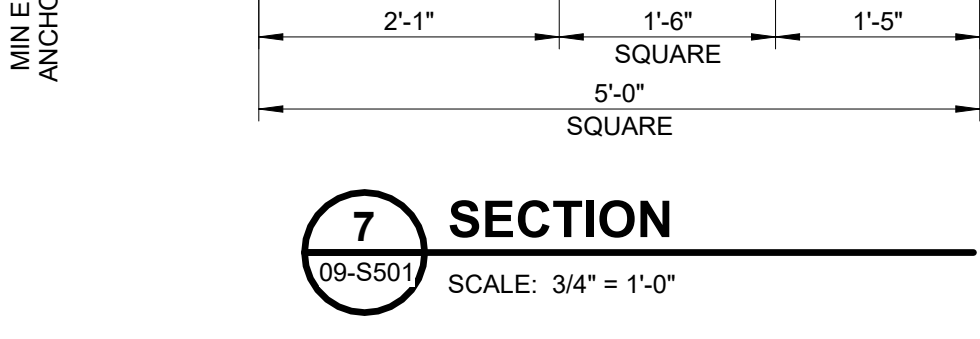
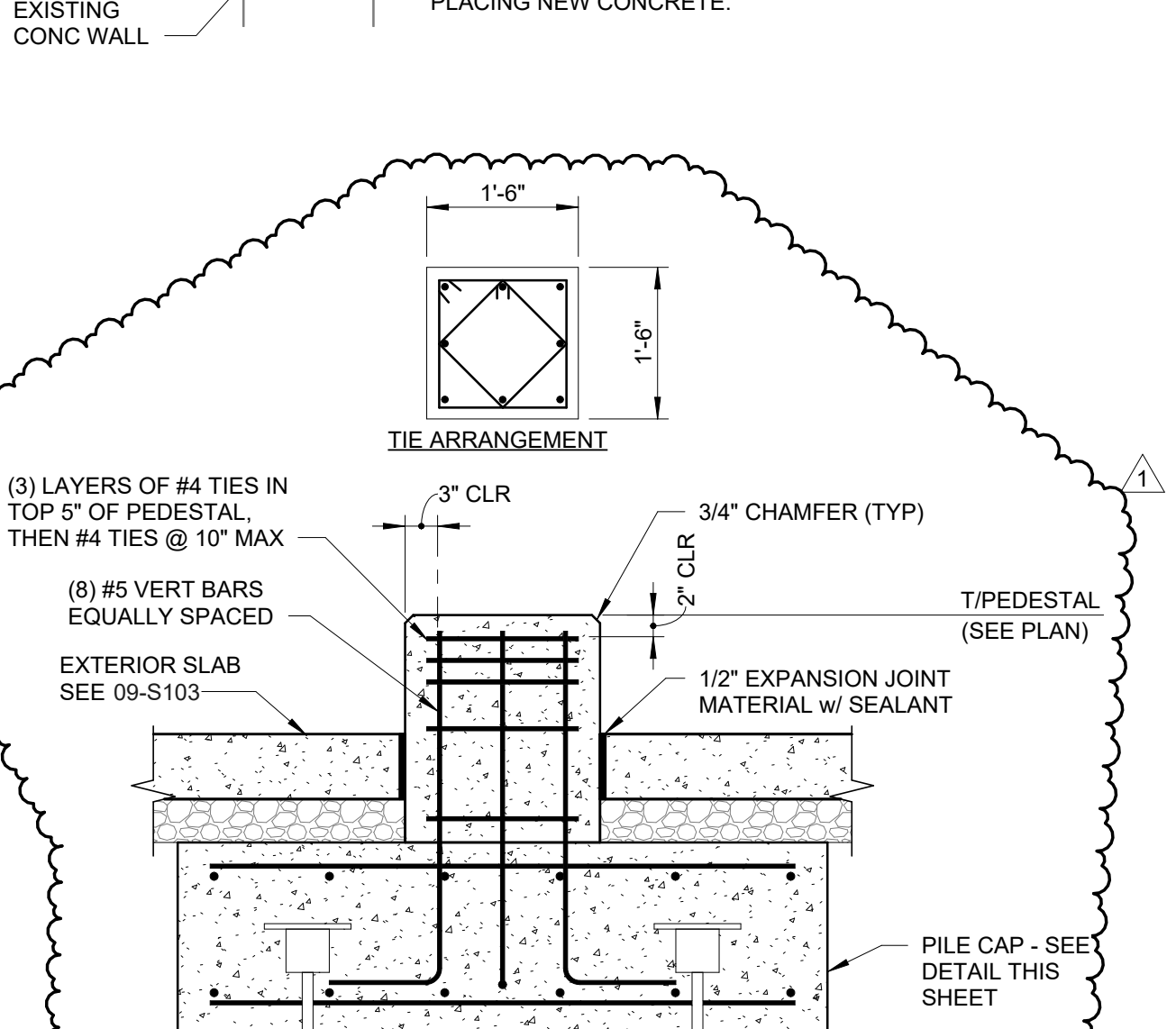
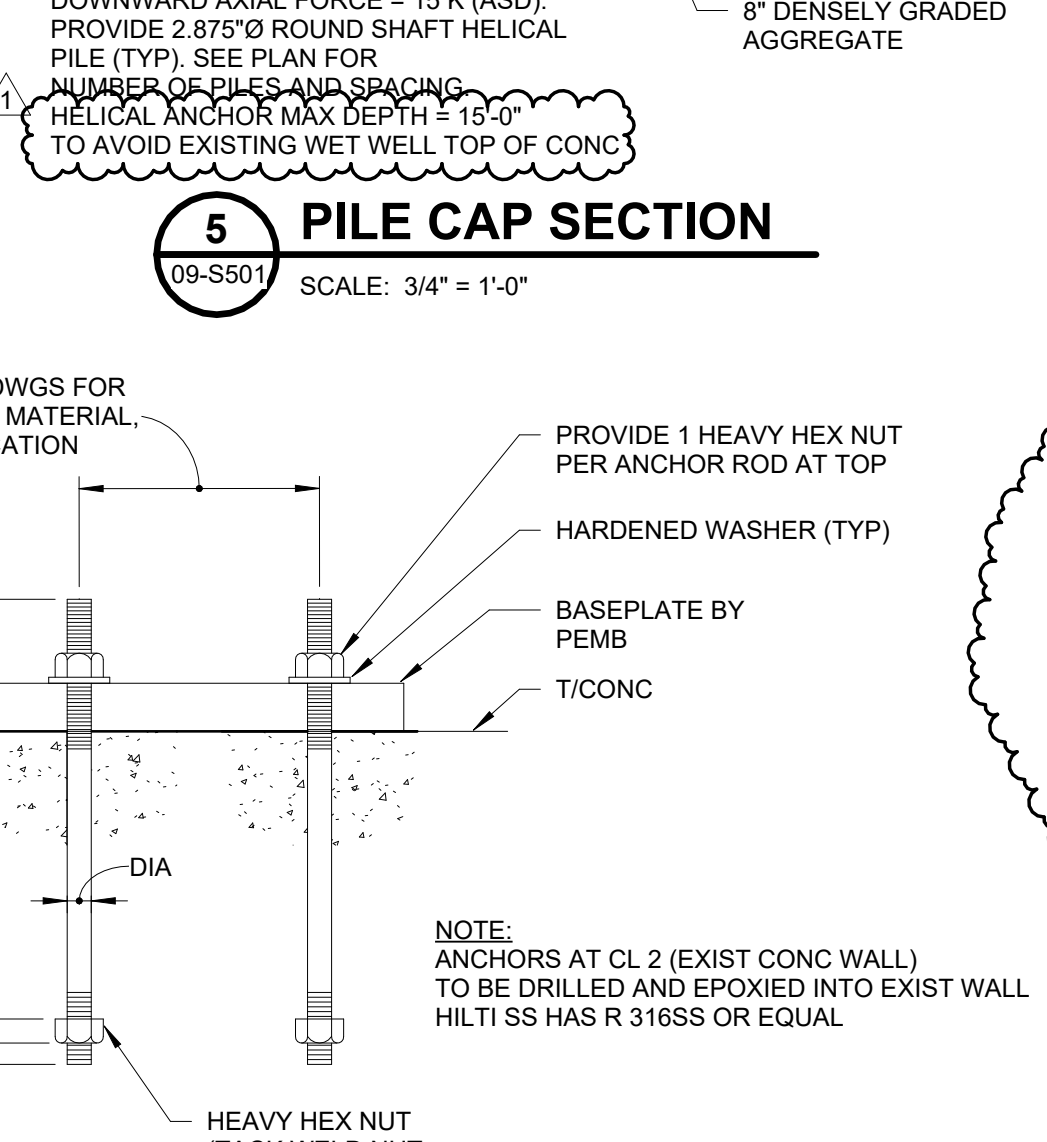
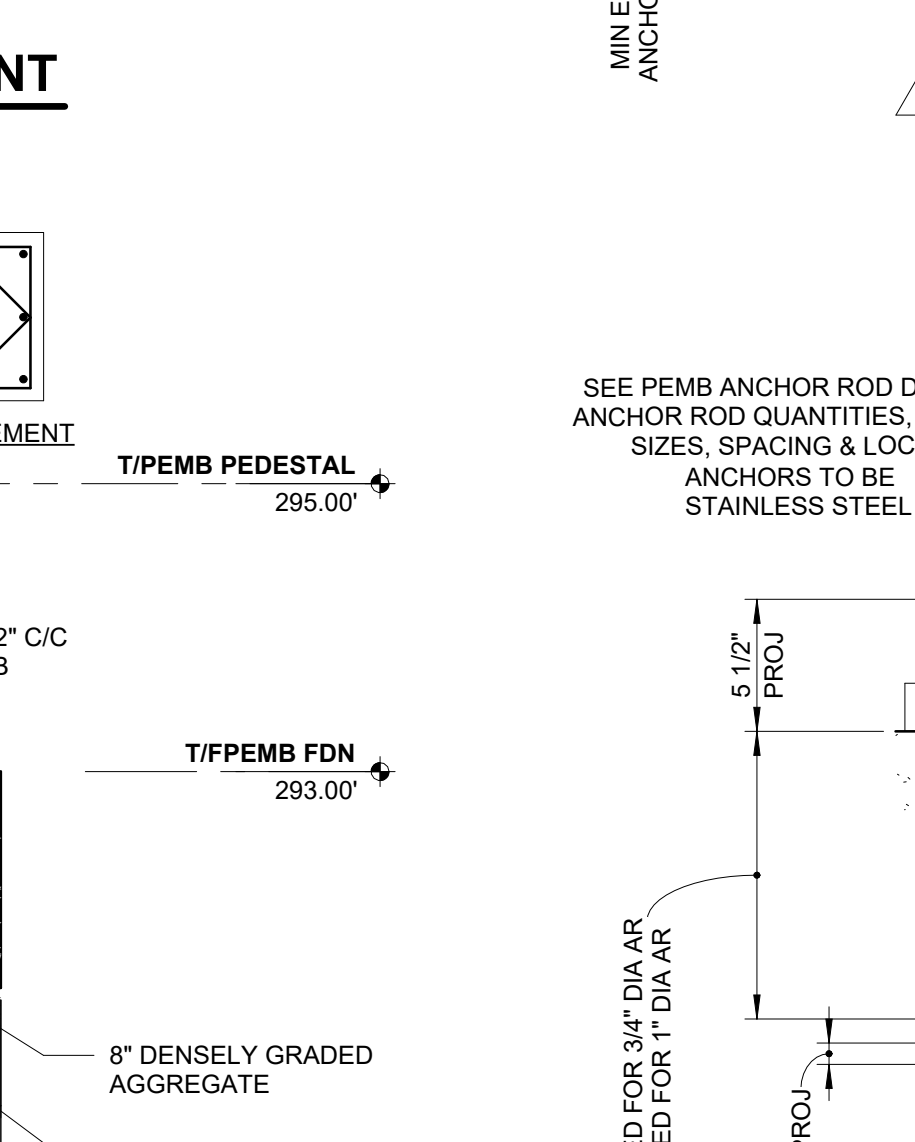
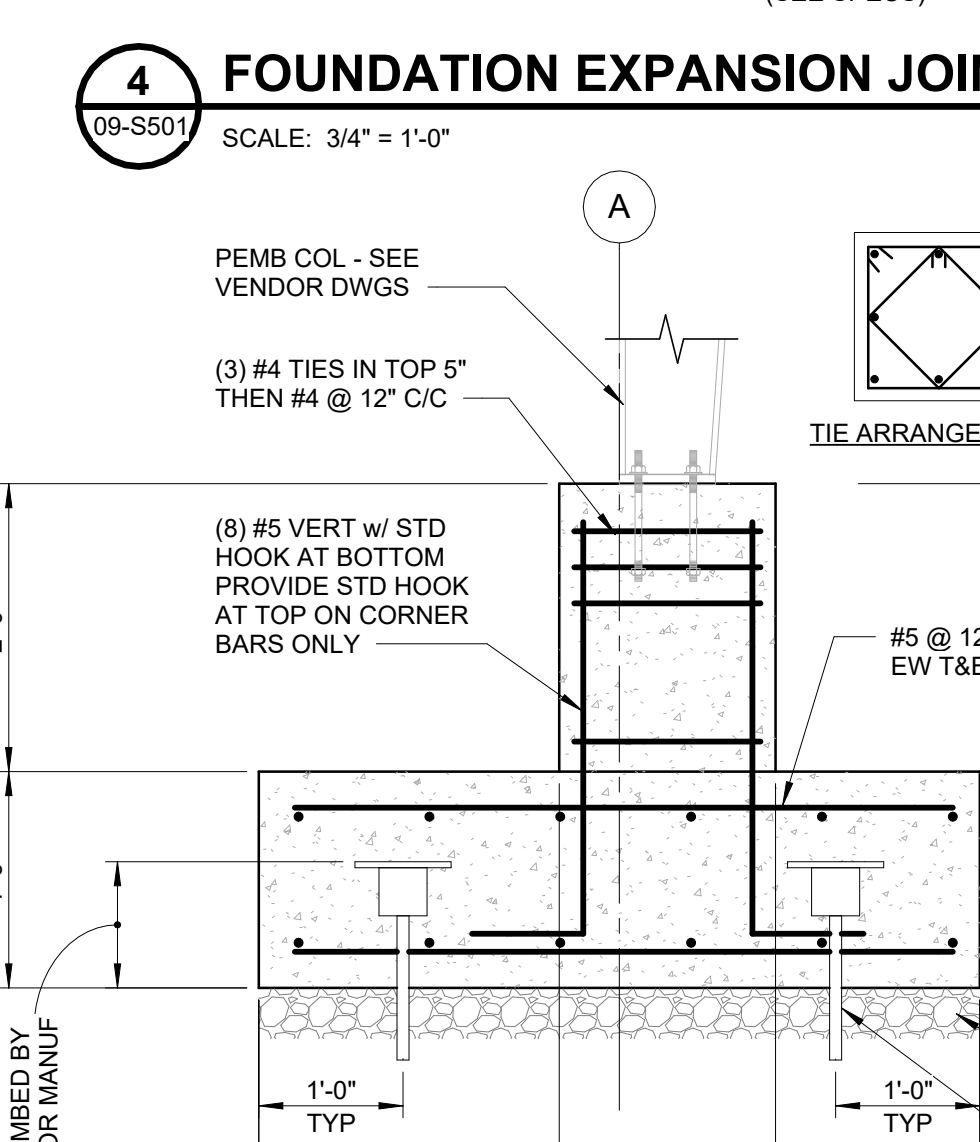
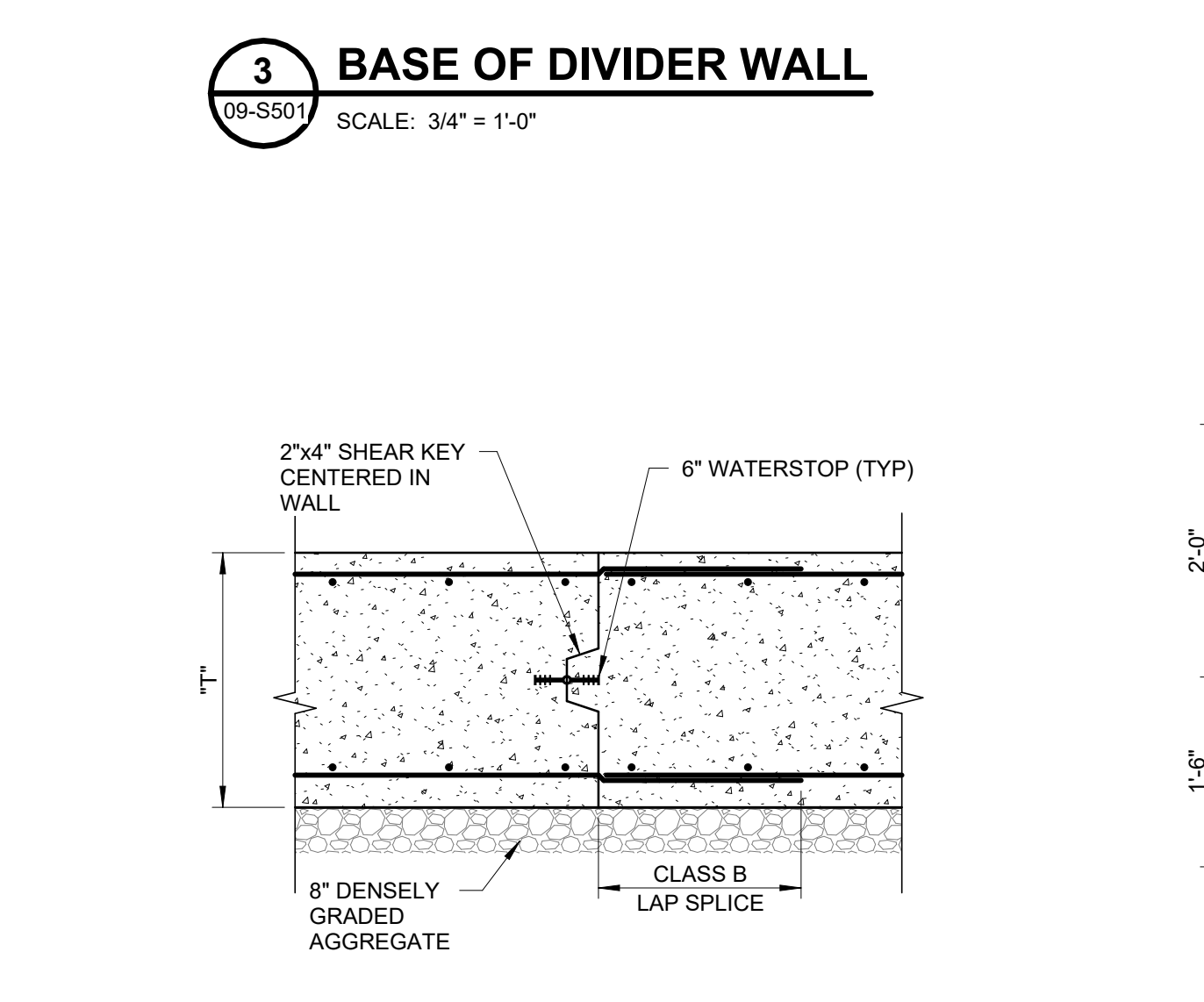
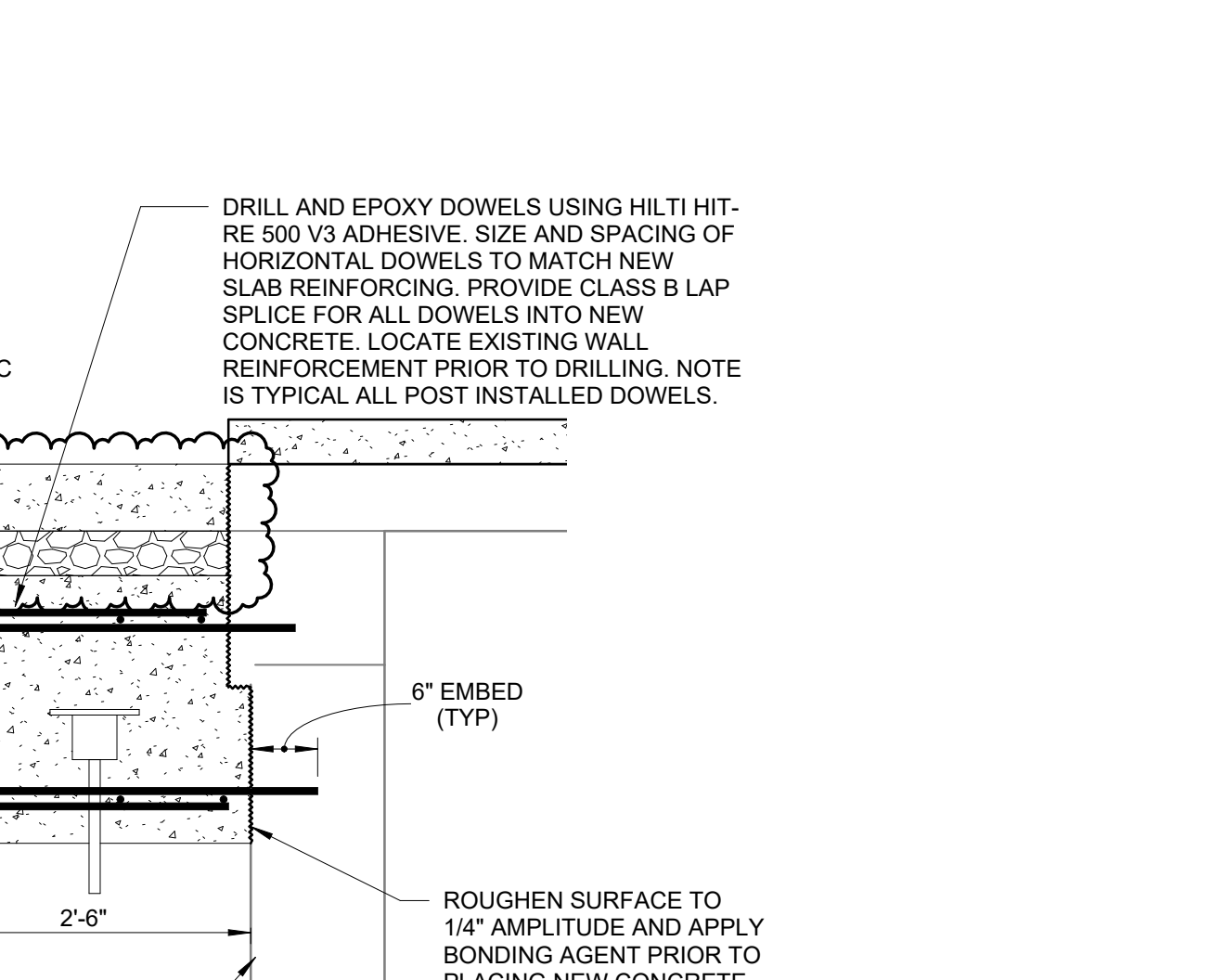
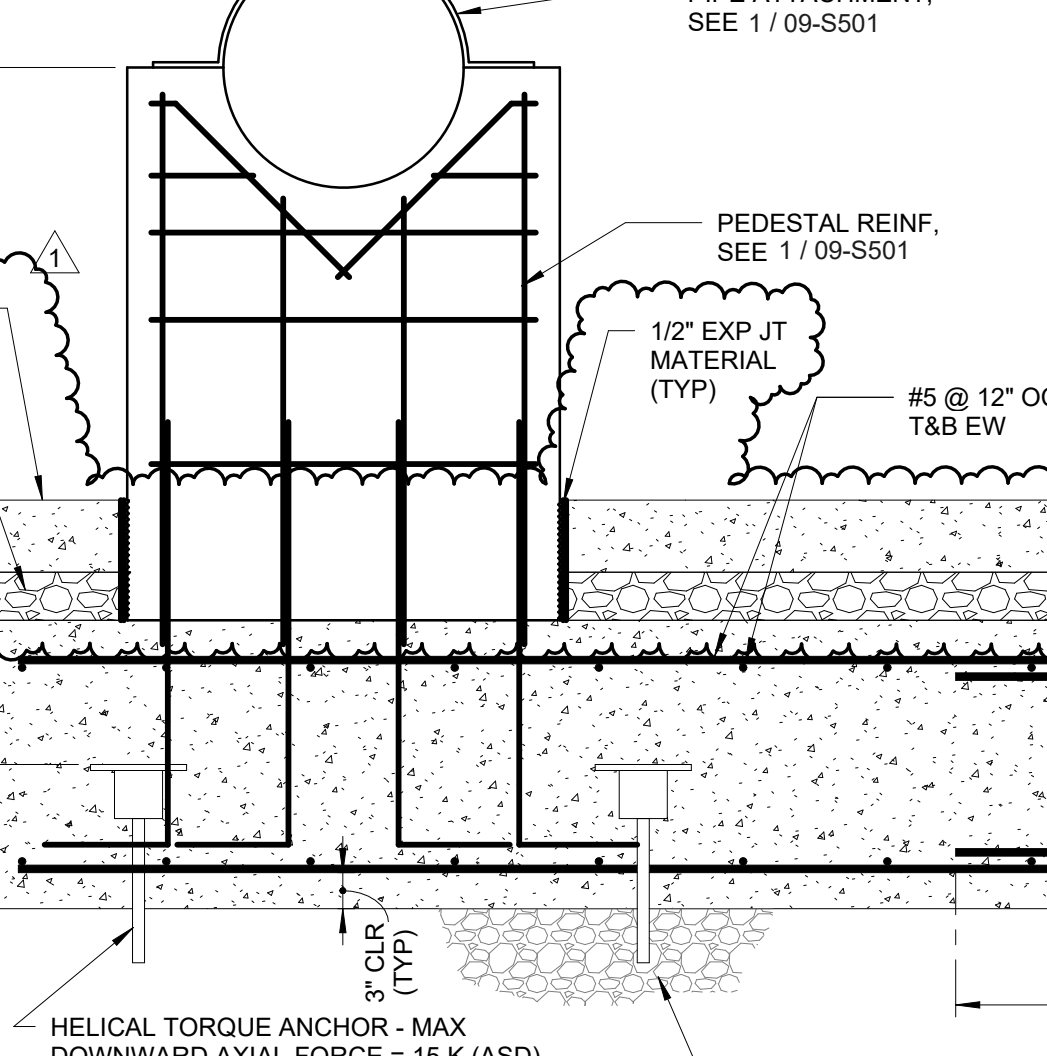
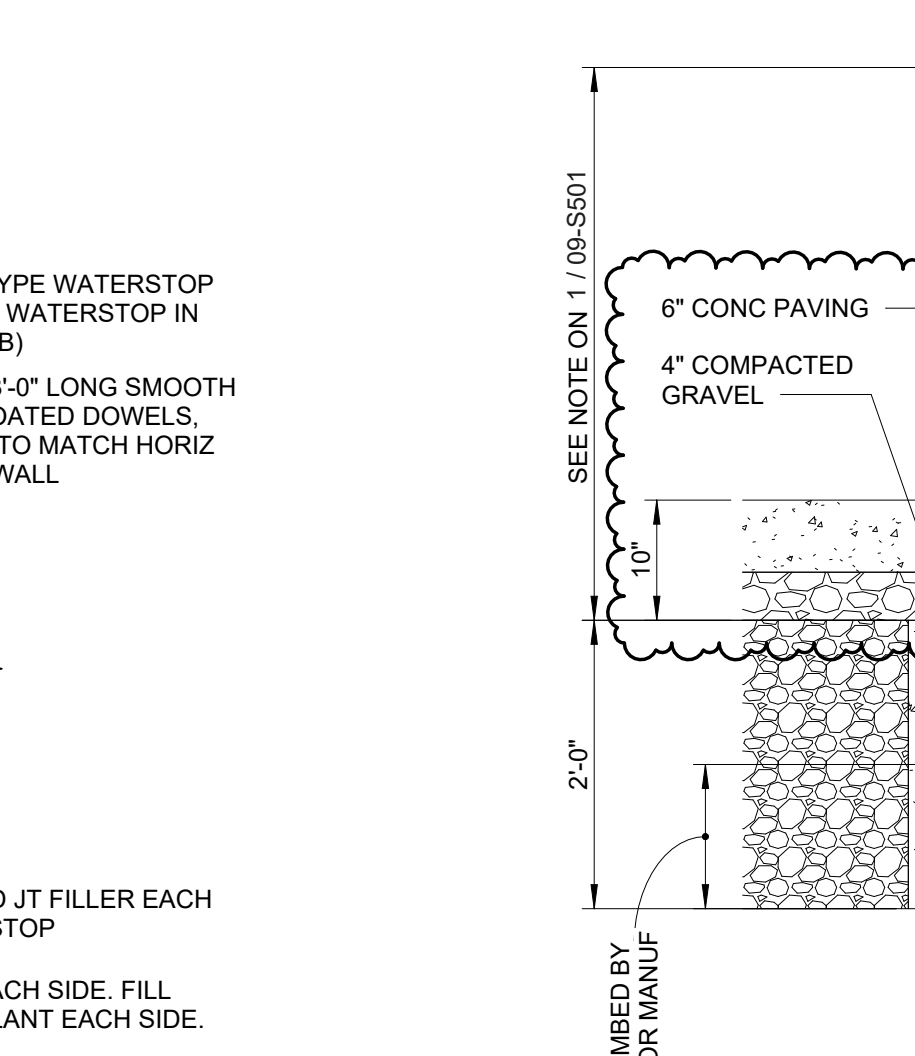
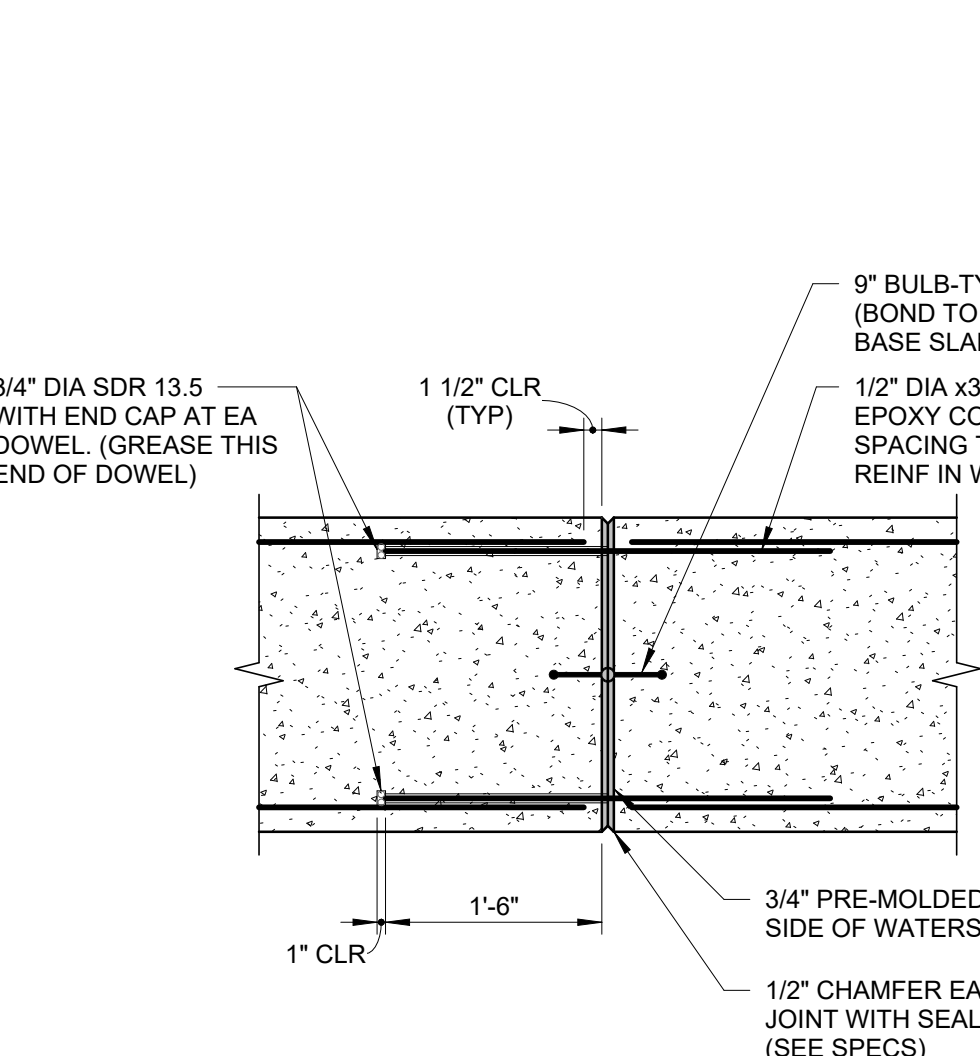
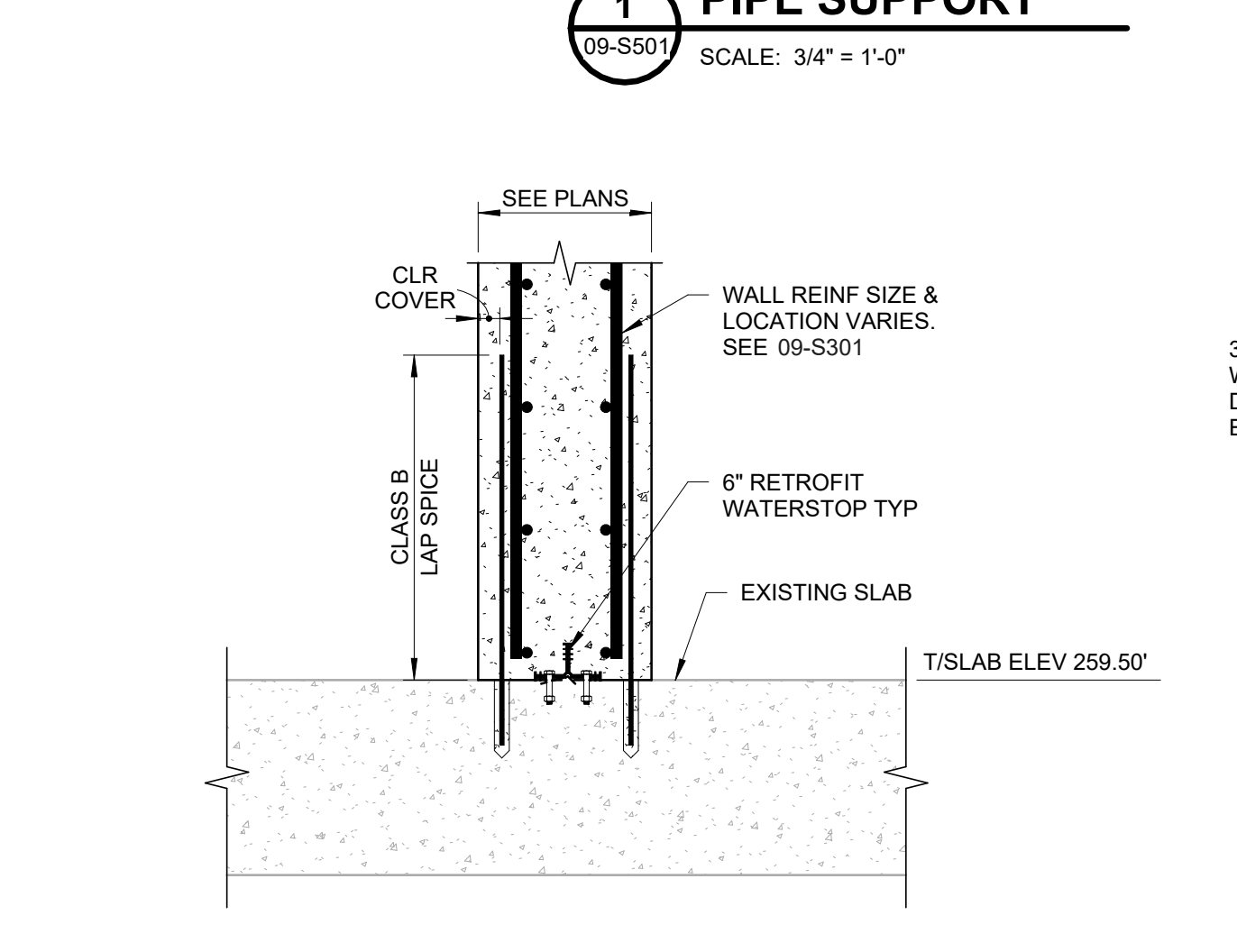
**3 BASE OF DIVIDER WALL**  
 09-S501 SCALE: 3/4" = 1'-0"

NOTE:  
 WALL REINF SIZE & LOCATION VARIES. SEE 09-S301



**4 FOUNDATION EXPANSION JOINT**  
 09-S501 SCALE: 3/4" = 1'-0"

NOTE:  
 PROVIDE MIN (2) 2.875" DIA ROUND SHAFT HELICAL TORQUE ANCHORS - MAX DOWNWARD FORCE = 15K (ASD) TYP



REVISION INFORMATION		DESCRIPTION:
CHK	JBA	ISSUED FOR BID
REV	0	07/10/2024
DR	MRD	08/14/2024
ACM	JBA	ADDENDUM 3